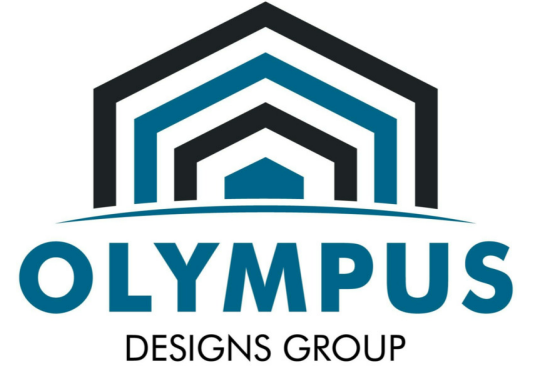


PROPERTY ADDRESS:
4311 15TH ST W LEHIGH ACRES FL 33971

LEGAL DESCRIPTION:
LEHIGH ACRES UNIT 9 BLK 81 PB 15 PG 79 LOT 2

STRAP: 28-44-26-09-00081.0020



ANY QUESTIONS REGARDING THE DRAWINGS, DESIGNS AND INFORMATION REPRESENTED HEREIN PLEASE CONTACT:

OLYMPUS DESIGNS GROUP

5621 STRAND BLVD, SUITE 308
NAPLES, FL 34110
239 306-2324
info@olympusdesigns.net

REVISIONS

| No. | Description | Date |
|-----|-------------|------|
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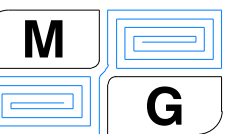
APPLICABLE CODES

-6TH EDITION (2017) FLORIDA BUILDING CODE
-6TH EDITION (2017) FLORIDA EXISTING CODE
-2014 STANDARD NATIONAL ELECTRICAL CODE

DIGITAL SEAL

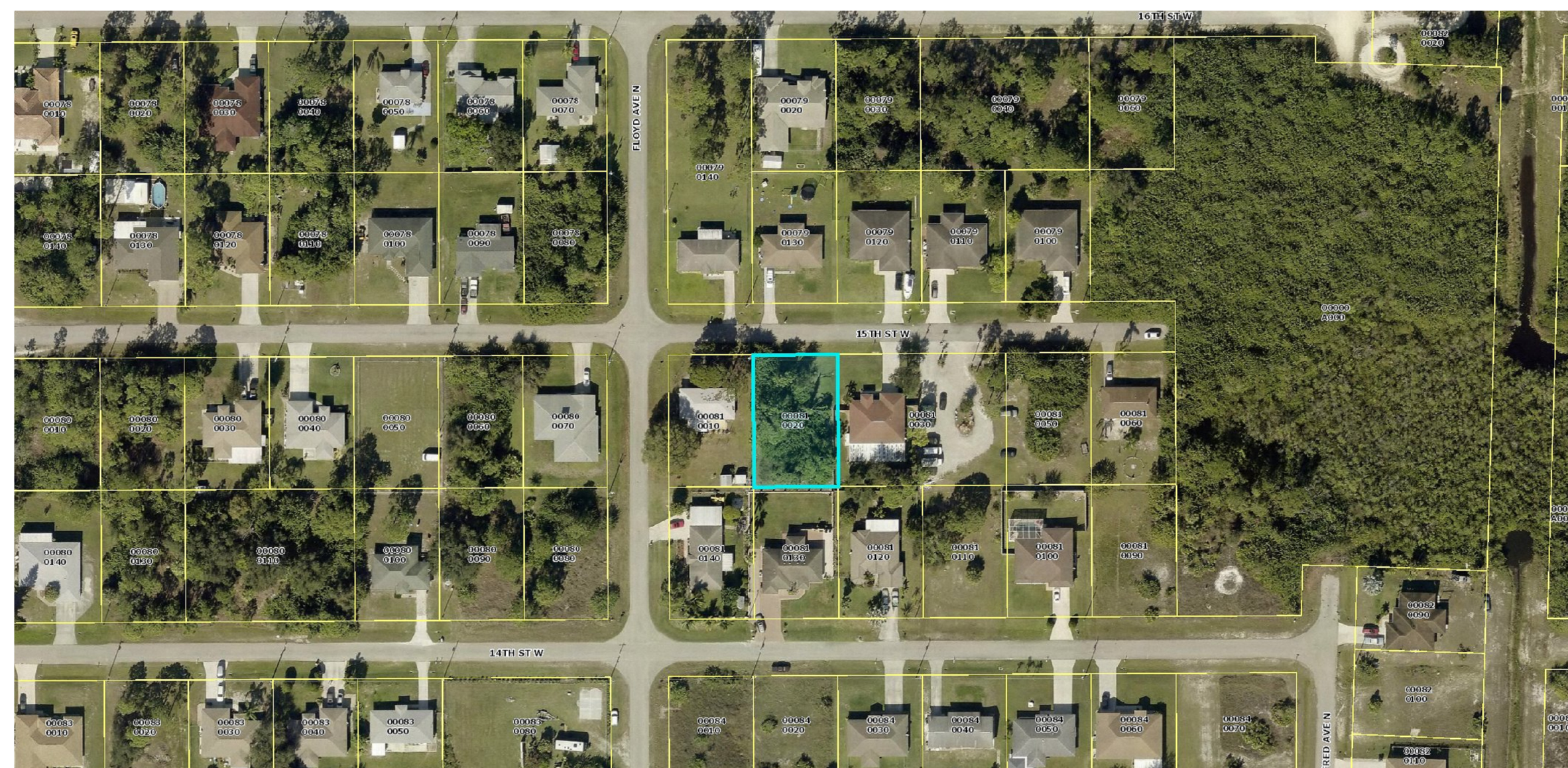
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3640 19th/ Ave SW
Naples, FL 34117
Phone: (239) 595 5465
maxguerra@mg-engineering.us



EXTERIOR PERSPECTIVE

SATELLITE VIEW LOCATION



| DRAWING INDEX | |
|---------------|-------------------|
| SHEET # | SHEET DESCRIPTION |
| CS | COVER SHEET |
| A1 | FLOOR PLAN |
| A2 | FOUNDATION PLAN |
| A3 | ELEVATIONS |
| A4 | ROOF/BEAMS PLAN |
| A5 | ELECTRICAL PLAN |
| A6 | DETAILS |

| AREA SUMMARY | |
|-----------------------|---------|
| ENTRY | 63 SF |
| COVERED LANAI | 125 SF |
| GARAGE | 393 SF |
| TOTAL A/C LIVING AREA | 1715 SF |
| TOTAL UNDER ROOF | 2297 SF |

MIRACLES LLC

4311 15TH ST W LEHIGH ACRES FL 33971

COVER SHEET

| | |
|----------------|-------|
| Project Number | |
| Date | 11/20 |
| Drawn by | IG |
| Notes | DS |

CS

Scale

JOB DATA

ULTIMATE DESIGN WIND SPEED : 160
 NOMINAL DESIGN WIND SPEED : 124
 RISK CATEGORY : 2
 IMPORTANCE FACTOR : 1.0
 BUILDING OCCUPANCY CLASSIFICATION : RESIDENTIAL
 BUILDING CONSTRUCTION TYPE : 5B
 EXPOSURE CATEGORY : B
 OPENING PROTECTION :
 INTERNAL PRESSURE COEFFICIENTS : +0.18, -0.18 (ENCLOSED)

FLOOR LIVE LOAD : 1/360 LIVE, 1/240 DEAD
 ROOF LIVE LOAD : 1/360 LIVE, 1/240 DEAD
 (SUPPORTING PLASTER CEILING)

THE STRUCTURAL COMPONENTS ON THIS PLANS ARE IN COMPLIANCE WITH THE 6TH EDITION FLORIDA BUILDING CODE, WIND LOAD COMPLIANCE AND ITS RESISTANCE TO GRAVITY AND DESIGN PRESSURES GENERATED BY A WIND VELOCITY OF 160 MPH

| | |
|-------------------------------|-------------|
| WIND VELOCITY (MPH) | 160 |
| IMPORTANCE FACTOR | 1.00 |
| EXPOSURE CATEGORY | B |
| INTERNAL PRESSURE COEFFICIENT | (+, -) 0.18 |
| MEAN ROOF HEIGHT (FT) | 15'-3" |
| BUILDING WIDTH (FT) | 49'-10" |
| BUILDING LENGTH (FT) | 53'-00" |
| ROOF SLOPE | 5:12 |
| ENCLOSED STRUCTURE | |

AREA SUMMARY

| | |
|-----------------------|---------|
| ENTRY | 63 SF |
| COVERED LANAI | 125 SF |
| GARAGE | 393 SF |
| TOTAL A/C LIVING AREA | 1715 SF |
| TOTAL UNDER ROOF | 2297 SF |

DOOR SCHEDULE

| Type Mark | Model | Width | Height | Type Comments | Max. Positive Pressure | Max. Negative Pressure |
|-----------|--------------|----------|---------|---------------|------------------------|------------------------|
| 1 | FRENCH DOOR | 5' - 4" | 8' - 0" | EXTERIOR | 41.8 | -46.2 |
| 2 | SLIDING DOOR | 8' - 0" | 8' - 0" | EXTERIOR | 40.6 | -44.5 |
| 3 | GARAGE | 16' - 0" | 8' - 0" | EXTERIOR | 38.9 | -43.3 |

INTERIOR DOOR SCHEDULE

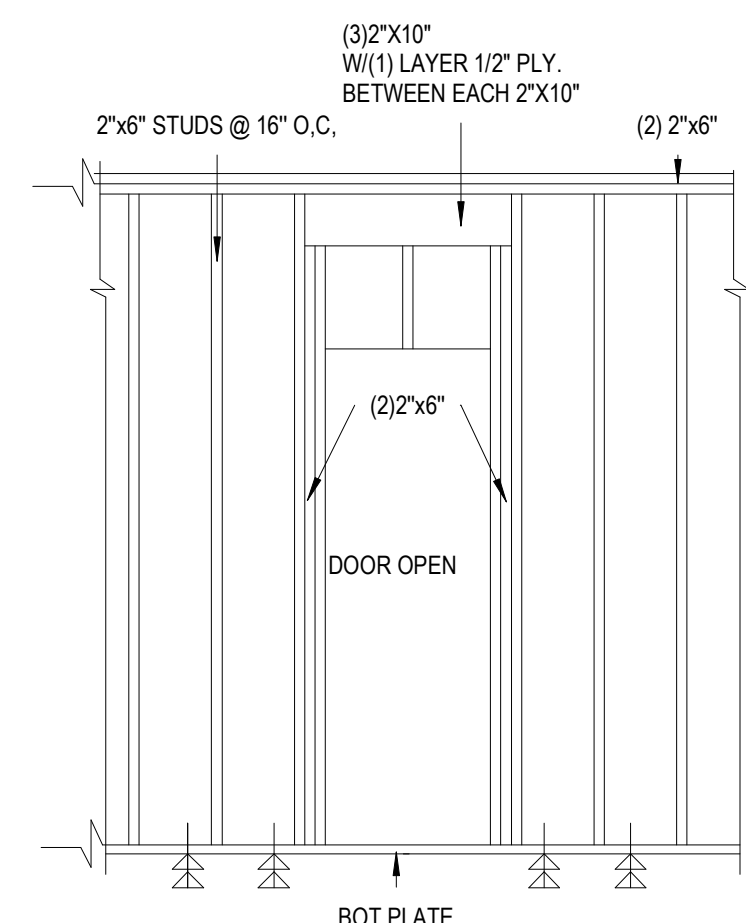
| Type Mark | Model | Width | Height | Type Comments | Count |
|------------|-----------------------|---------|---------|---------------|-------|
| (2) F-3068 | BI-FOLD | 6' - 0" | 6' - 8" | INTERIOR | 2 |
| 2868 | SWING | 2' - 8" | 6' - 8" | INTERIOR | 4 |
| F-2480 | BI-FOLD | 2' - 4" | 8' - 0" | INTERIOR | 2 |
| SD-2880 | SLIDING DOOR (POCKET) | 2' - 6" | 8' - 0" | INTERIOR | 2 |

WINDOW SCHEDULE

| Type Mark | Width | Height | Description | Count | Max. Positive Pressure | Max. Negative Pressure |
|-----------|---------|---------|-------------|-------|------------------------|------------------------|
| 25 | 3' - 0" | 5' - 2" | SINGLE HUNG | 5 | | |
| 35 | 4' - 5" | 5' - 2" | SINGLE HUNG | 1 | 43.6 | -47.5 |
| 77 | 5' - 0" | 1' - 4" | FIXED | 1 | 46.1 | -50 |
| 78 | 2' - 2" | 2' - 2" | SINGLE HUNG | 1 | 46.1 | -50 |
| 79 | 3' - 0" | 3' - 0" | SINGLE HUNG | 2 | 46.1 | -50 |

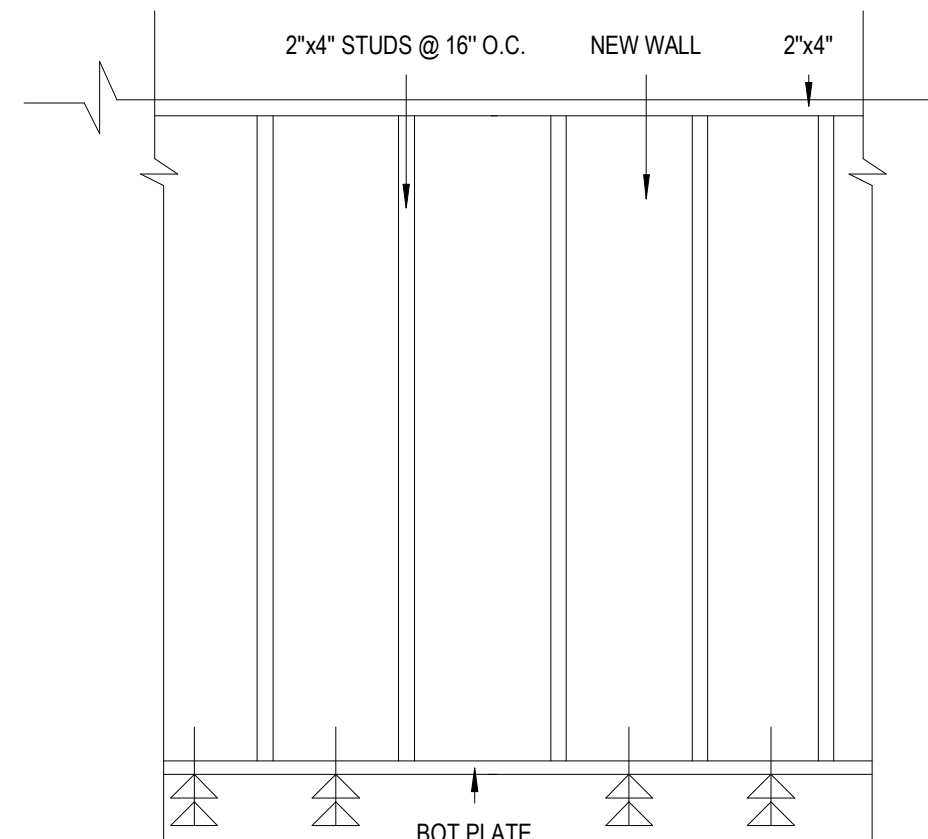
NOTES:

- CONTRACTOR TO VERIFY WINDOWS AND DOORS ROUGH OPENINGS AS PER THE MANUFACTURER SPECIFICATIONS.
- OPENINGS BETWEEN THE GARAGE AND RESIDENCE SHALL BE EQUIPPED WITH SOLID WOOD DOORS NOT LESS THAN 1-3/8 INCHES (35 MM) IN THICKNESS, SOLID OR HONEYCOMB-CORE STEEL DOORS NOT LESS THAN 1-3/8 INCHES (35 MM) THICK, OR 20 MINUTE FIRE-RATED DOORS. (R302.5.1)
- THE GARAGE SHALL BE SEPARATED FROM THE RESIDENCE AND ITS ATTIC AREA BY NOT LESS THAN A 1/2-INCH (12.7 MM) GYPSUM BOARD OR EQUIVALENT APPLIED TO THE GARAGE SIDE. THE GARAGE SHALL BE SEPARATED FROM ALL HABITABLE ROOMS ABOVE, BY NOT LESS THAN 5/8-INCH (15.9 MM), TYPE X GYPSUM BOARD OR EQUIVALENT. (R302.6)

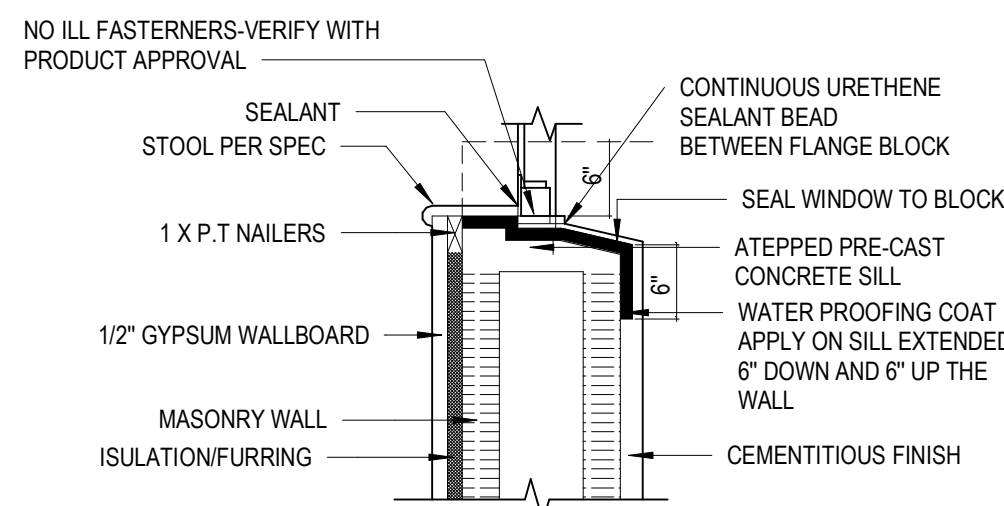


TYP. DETAIL BEARING WALL
N.T.S

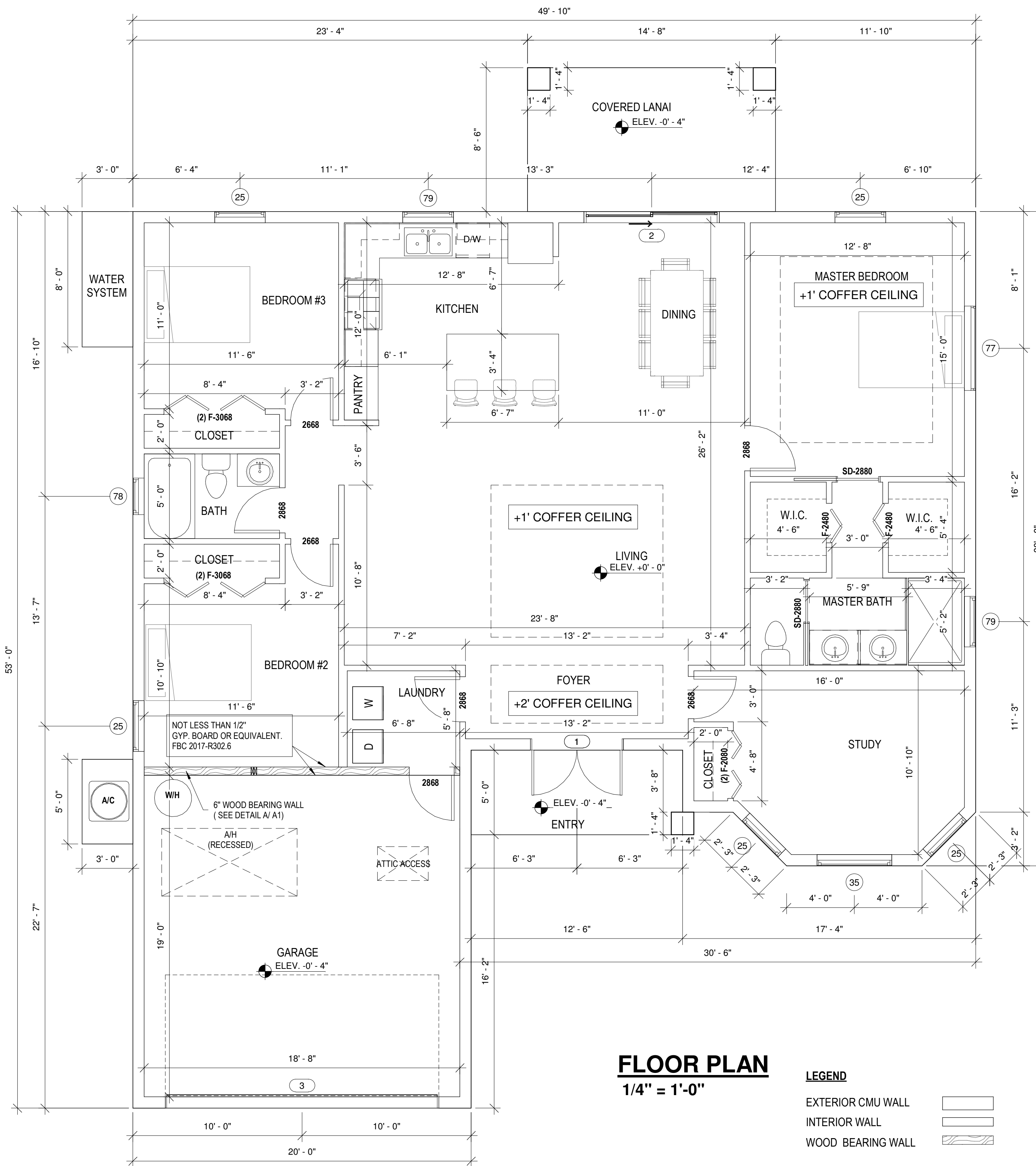
DETAIL A
N.T.S



TYP. DETAIL NEW NON BEARING WALL
N.T.S



SH WINDOW SILL-CMU
N.T.S



FLOOR PLAN
1/4" = 1'-0"

LEGEND

- EXTERIOR CMU WALL
- INTERIOR WALL
- WOOD BEARING WALL

NOTE A:
 TABLE 722.3.2
 MINIMUM EQUIVALENT THICKNESS (in)
 OF BEARING OR NONBEARING CONCRETE MASONRY WALLS: 2.8
 TYPE OF AGGREGATE: CALCAREOUS OR SILICEOUS GRAVEL
 FIRE-RESISTANCE RATING (hours): 1



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 239 306-2324
 info@olympusdesigns.net

REVISIONS

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MIRACLES LLC

4311 15TH ST W LEHIGH ACRES FL 33971

FLOOR PLAN

| | |
|----------------|-------|
| Project Number | |
| Date | 11/20 |
| Drawn by | IG |
| Notes | DS |

A1

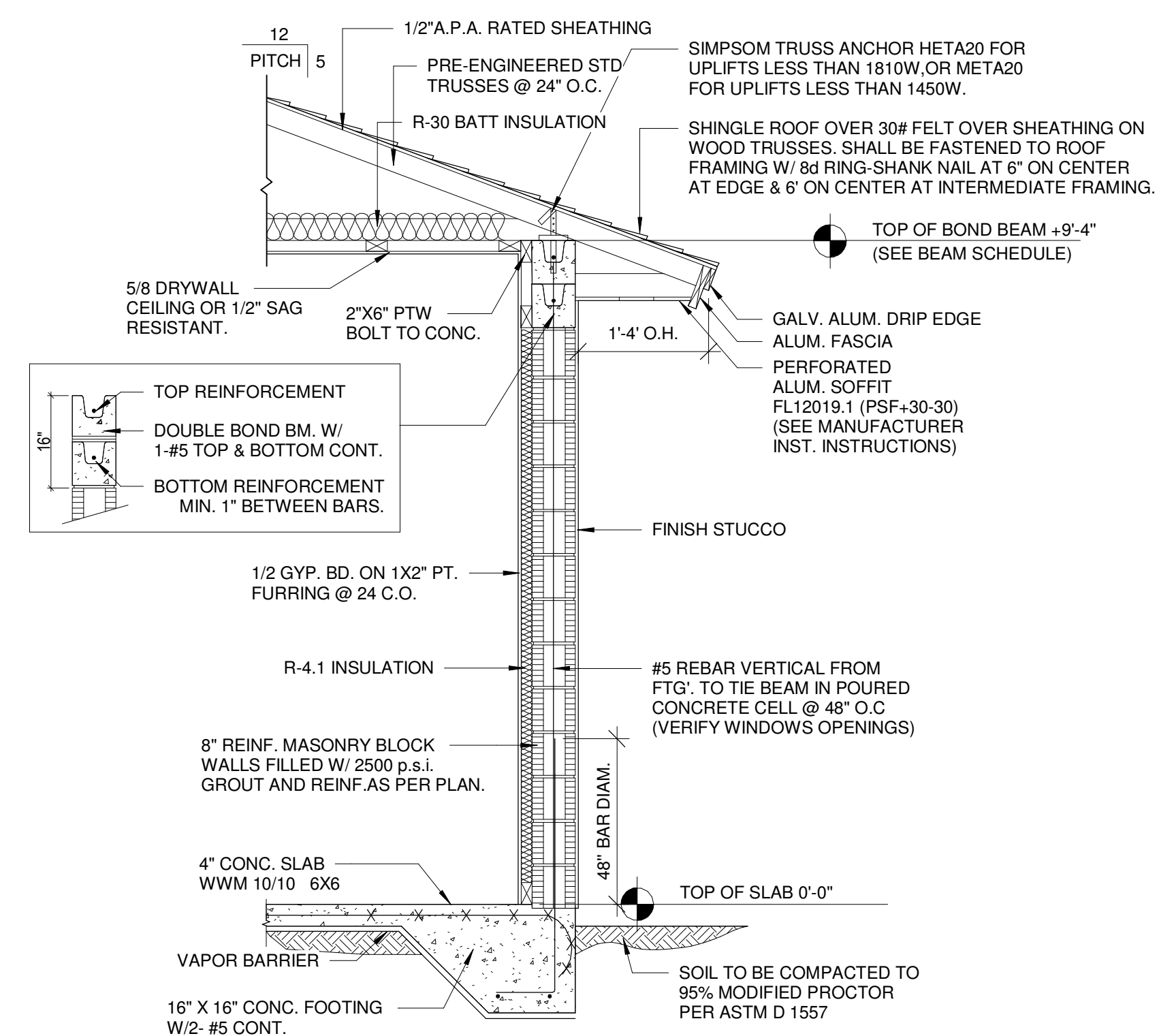
Scale As indicated

FOUNDATION GROUND FLOOR NOTES

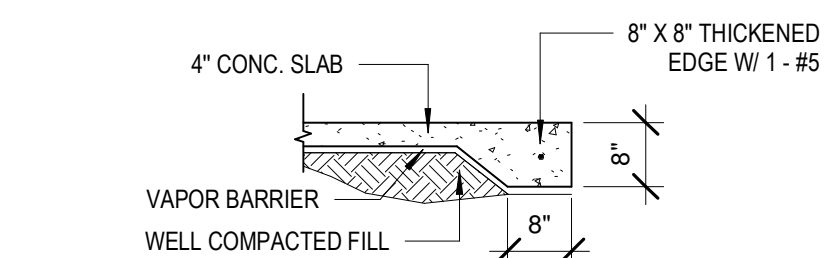
- FLOOR SLAB IS A 4" CONC. SLAB-ON-GRADE (fc=3000 psi) WITH 6 X 6 W1.4 X W1.4 W.W.F. @ MID-DEPTH (NOT SHOWN) ON WELL COMPACTED & TREATED SOIL OVER 6 MIL. VAPOR BARRIER. REFER TO DETAIL. SOIL SHALL BE COMPACTED TO 95% MODIFIED PROCTOR PER ASTM D 1557 IN LIFTS NOT EXCEED 12".
- FOUNDATIONS ARE DESIGNED FOR 3000 PSF. GENERAL CONTRACTOR SHALL VERIFY THE VALIDITY OF THIS ASSUMPTION.
- CENTER OF LOAD SHALL COINCIDE WITH CENTER OF FOOTING U.N.O.
- ALL CONCRETE TO HAVE A MINIMUM OF 3000 PSI COMPRESSIVE STRENGTH WITH THE WATER/CEMENT RATIO OF 0.5 MAXIMUM.
- INDICATES ADDITIONAL #5 IN CMU WALLS.
- REFER TO ARCHITECTURE PLANS FOR ALL DIMENSIONS.
- ALL REINFORCEMENT SHALL BE GRADE 60.
- BUILDING PAD SHALL BE COMPACTED 5' - 0" EACH SIDE OF THE BUILDING AND SLOPE AT 30 DEGREES TO GRADE. FOOTING SHALL EMBED AT LEAST 12". IF THESE CONDITIONS CAN NOT BE MET, THE BUILDING SHALL BE ON STEM WALL FOUNDATION. NOTIFY ENGINEER PRIOR TO CONSTRUCTION.
- SOIL SHALL BE TREATED FOR TERMITES PROTECTION AS PER SECTION R318 OF F.B.C 6TH EDITION 2017.

MASONRY WALL REINFORCEMENT CEMENT

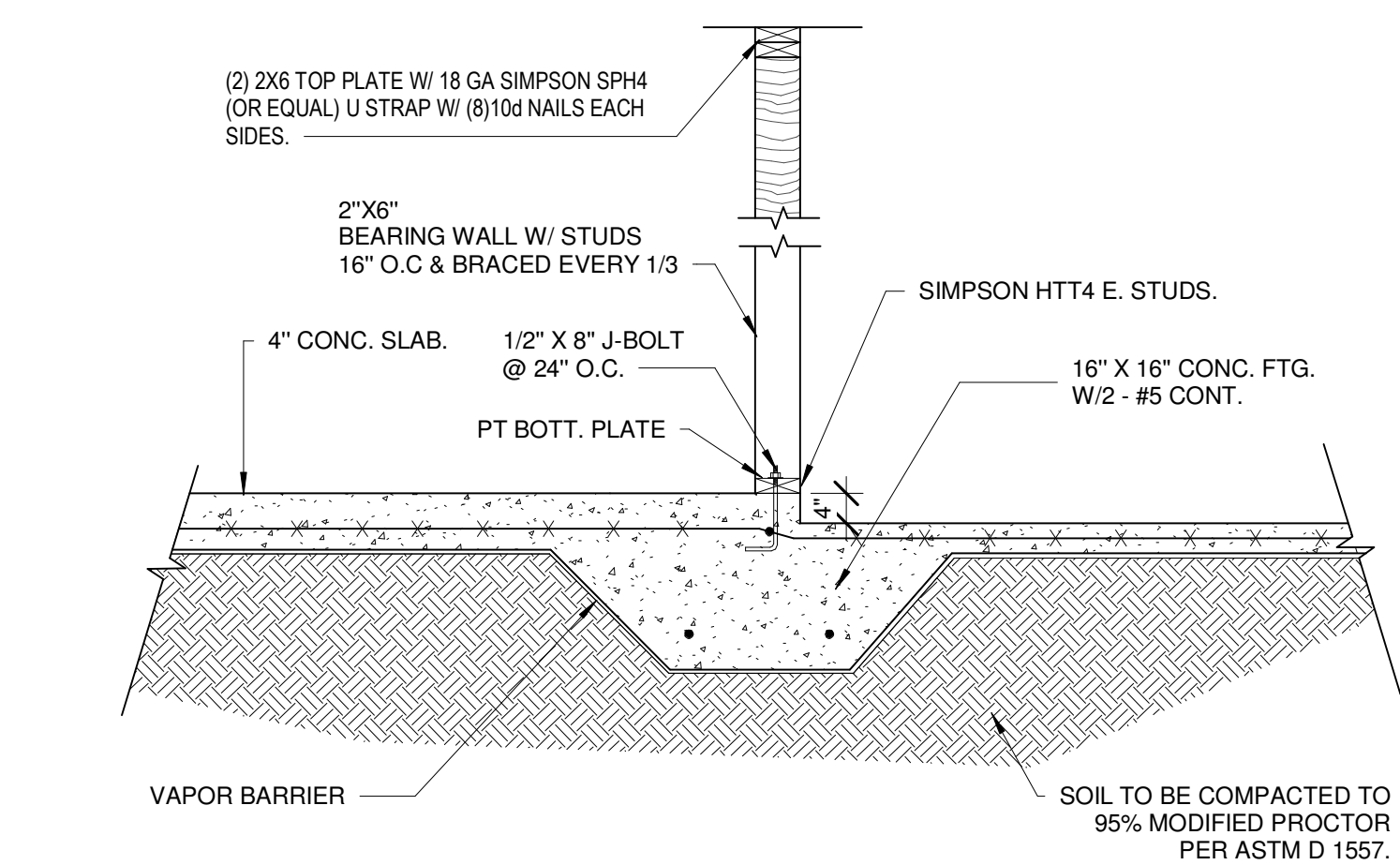
- WALL REINFORCEMENT SHALL BE DOWELED FROM FOUNDATION AND BE CONTINUOUS THROUGH SOLID GROUTED CELLS AND BE HOOKED OVER TOP REINFORCEMENT OF UPPER BEAMS, MINIMUM LAP SPLICE SHALL BE 48 BAR DIAMETERS. FOR HORIZONTAL WALL REINFORCEMENT, @ EVERY OTHER COURSE.
- WALL REINFORCEMENT IS FOLLOWS: #5 @ 48" O.C. PROVIDE 1 #5 AT ALL WALL INTERSECTIONS, CORNERS, AND EACH SIDE OF OPENINGS AND 2 #5 EACH SIDE OF OPENINGS LARGER THAN 6' - 0".
- WALL SEGMENT BELOW AND ABOVE THE OPENINGS SHALL BE REINFORCED SAME AS WALL.
- MASONRY GROUT = 2500 P.S.I.
- MASONRY WALL COMPRESSIVE STRENGTH OF fm = 1500 P.S.I.
- MORTAR TYPE M OR S WITH 1900 P.S.I COMPRESSIVE STRENGTH.



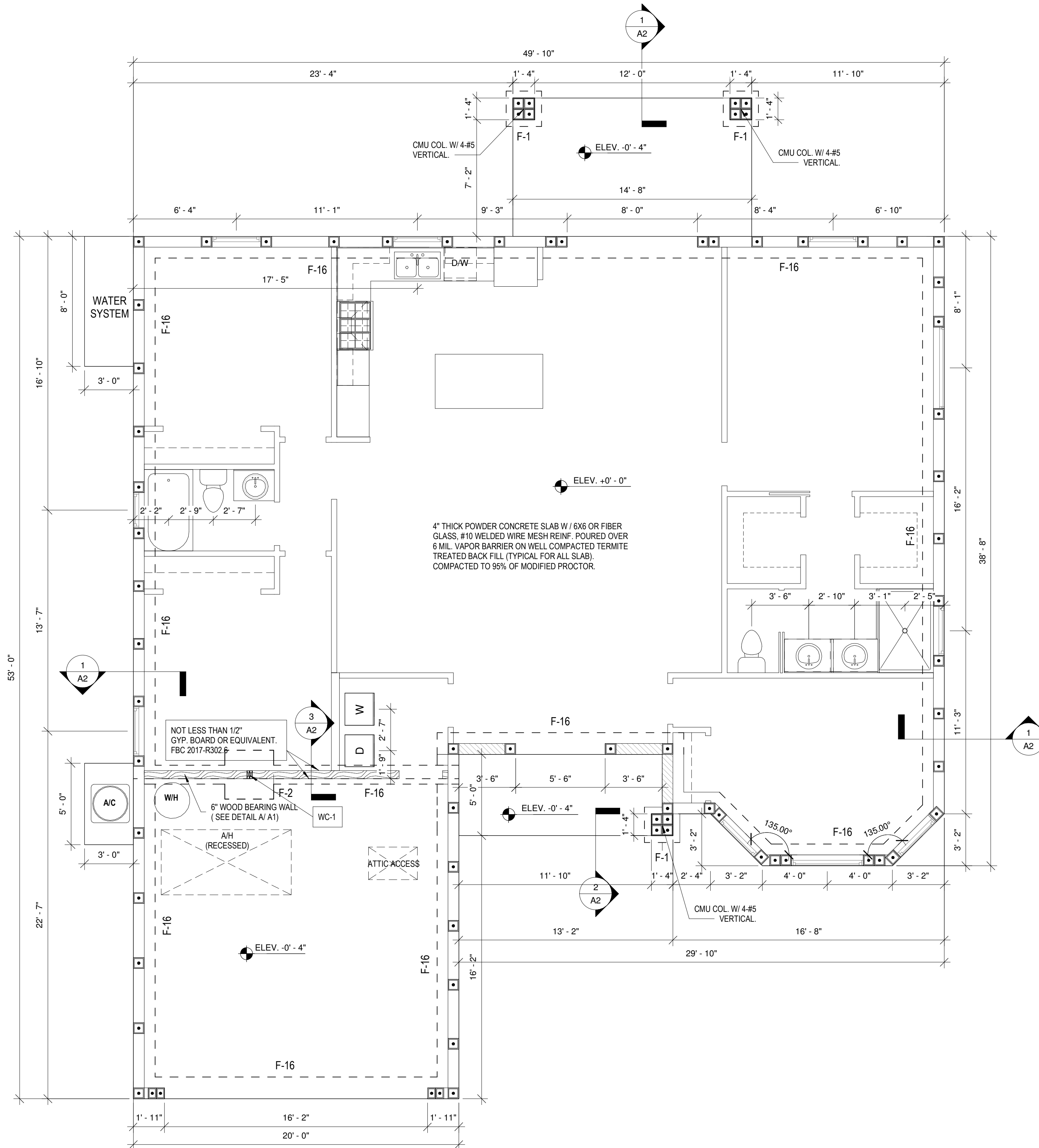
1 TYP. WALL SECTION
1/2" = 1'-0"



2 THICKENED EDGE
1/2" = 1'-0"



3 WOOD BEARING WALL
1/2" = 1'-0"



FOUNDATION PLAN
1/4" = 1'-0"

LEGEND
WALL FOOTING ---
8" REINF. MASONRY BLOCK WALL. [Symbol]

| FOOTING SCHEDULE | | |
|------------------|-----------------|------------------------------------|
| MARK | SIZE | REINFORCEMENT |
| F-1 | 26" X 26" X 16" | W/4 - #5 EA. WAY, BOT. |
| F-2 | 30" X 30" X 16" | W/5 - #4 EA. WAY, BOT. |
| F-16 | 16" X 16" | 16"X16" CONC. FOOTING W/2-#5 CONT. |

| WOOD COLUMN SCHEDULE | |
|----------------------|--|
| Type Mark | Description |
| WC-1 | (2)2"x6" W/ SIMPSON (1) HTT4 BOT & (2)HTS20 TOP. |



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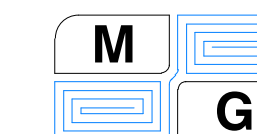
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MIRACLES LLC

4311 15TH ST W LEHIGH ACRES FL 33971

FOUNDATION PLAN

Project Number
Date **11/20**
Drawn by **IG**
Notes **DS**

A2

Scale **As indicated**



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MIRACLES LLC

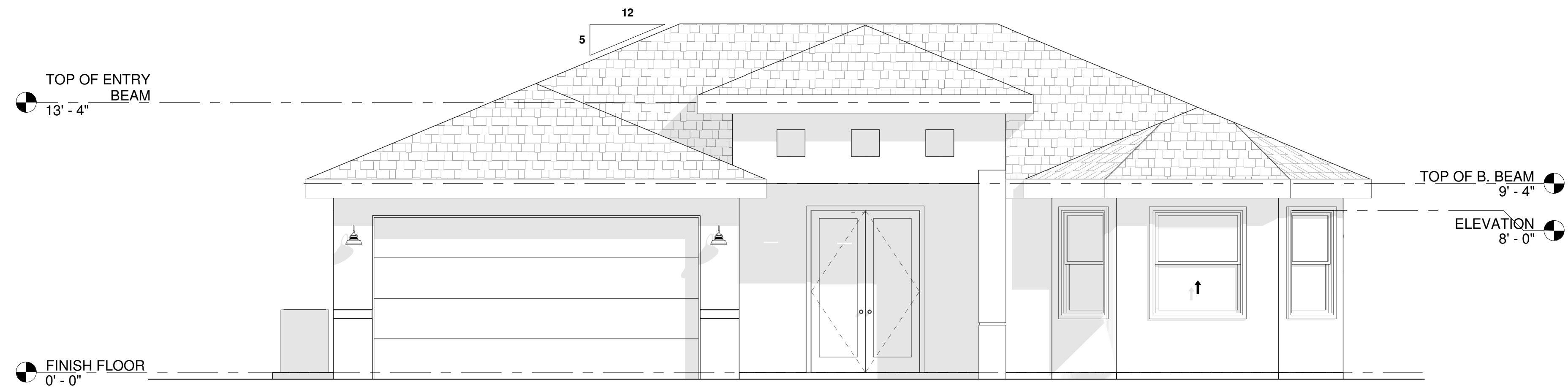
4311 15TH ST W LEHIGH ACRES FL 33971

ELEVATIONS

| | |
|----------------|-------|
| Project Number | |
| Date | 11/20 |
| Drawn by | IG |
| Notes | DS |

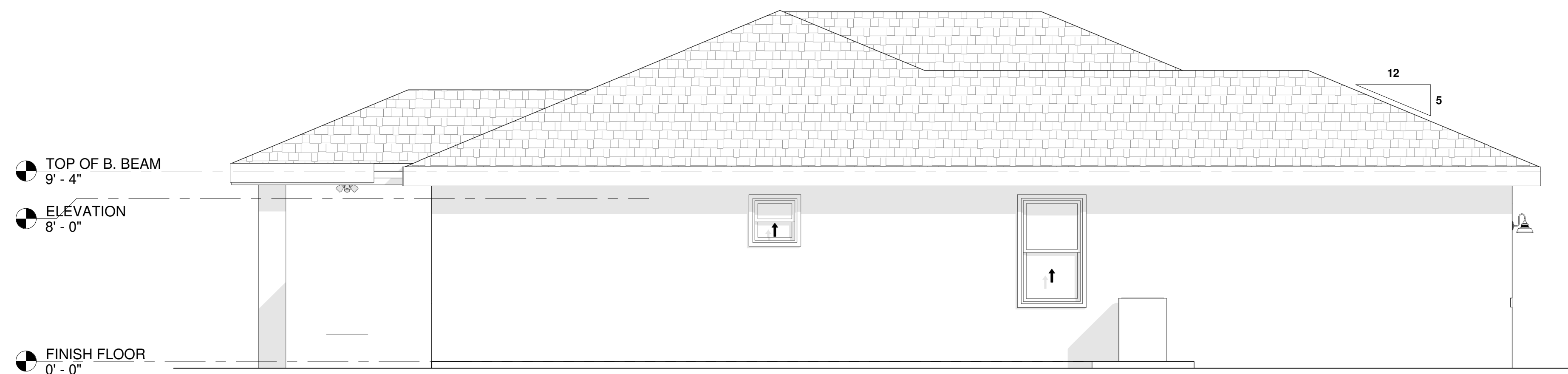
A3

Scale 1/4" = 1'-0"



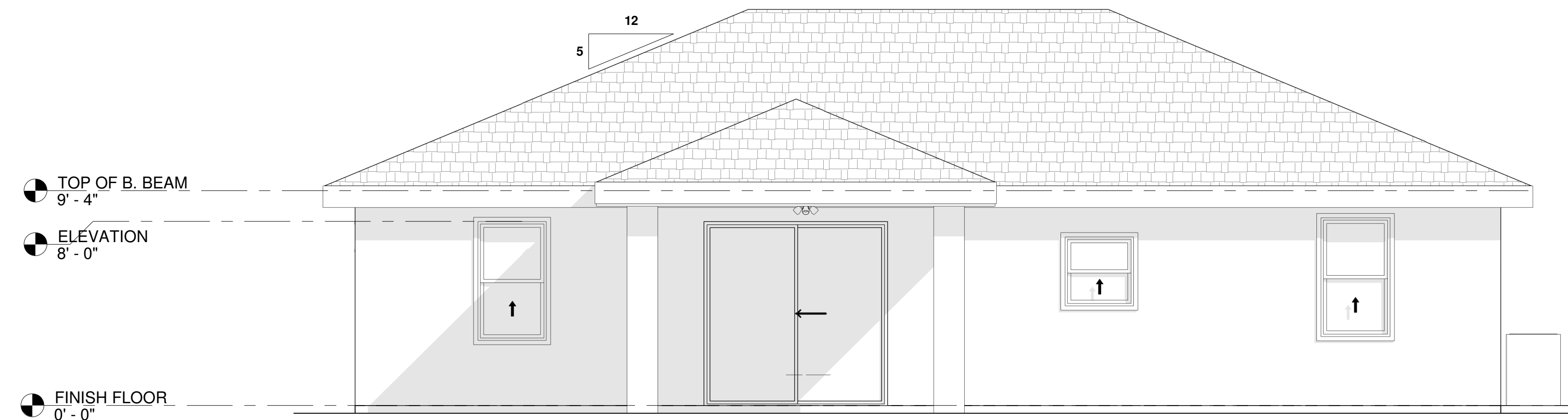
FRONT ELEVATION

1/4" = 1'-0"



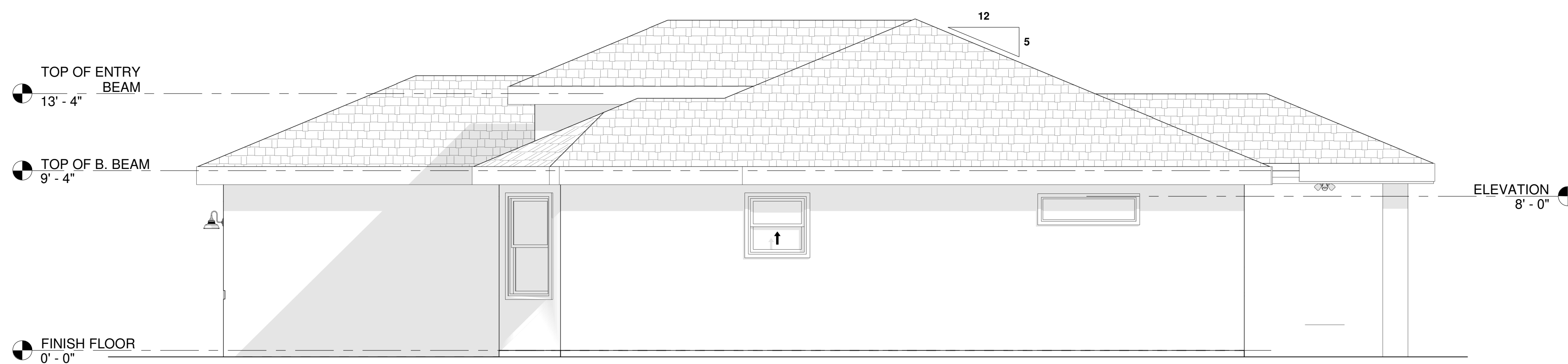
RIGHT ELEVATION

1/4" = 1'-0"



REAR ELEVATION

1/4" = 1'-0"



LEFT ELEVATION

1/4" = 1'-0"

MINIMUM ROOF VENT AREA PER FBC R806.2
 AREA: 2297 SQ. FT.
 VENTING REQUIRED: 2297 X 144 SQ. IN./SQ. FT. X 1/300 SQ. IN. VENT/SQ. IN. ROOF = 1102 SQ. IN.

NOTE:
 1/150 IS REQUIRED UNLESS AT LEAST 40 PERCENT AND NOT MORE THAN 50 PERCENT OF THE REQUIRED VENTILATING AREA IS PROVIDED BY VENTILATORS LOCATED IN THE UPPER PORTION OF THE ATTIC OR RAFTER SPACE. UPPER VENTILATORS SHALL BE LOCATED NO MORE THAN 3 FEET (914 mm) BELOW THE RIDGE OR HIGHEST POINT OF THE SPACE, MEASURED VERTICALLY WITH THE BALANCE OF THE REQUIRED VENTILATION PROVIDED BY EAVE OR CORNICE VENTS. WHERE THE LOCATION OF WALL OR ROOF FRAMING MEMBERS CONFLICTS WITH THE INSTALLATION OF UPPER VENTILATORS, INSTALLATION MORE THAN 3 FEET (914 mm) BELOW THE RIDGE OR HIGHEST POINT OF THE SPACE SHALL BE PERMITTED.

DISCRETE ROOF AREAS TO HAVE PERCENTAGE OF TOTAL VENTING PROPORTIONAL TO THE PERCENTAGE OF AREA. SMALL ROOF AREAS SUCH AS ENTRY TOWERS MAY BE VENTED W/ SOFFIT VENTS ONLY PROVIDED VENTILATION IS BASED ON THE 2X INCREASE IN NET FREE AREA.

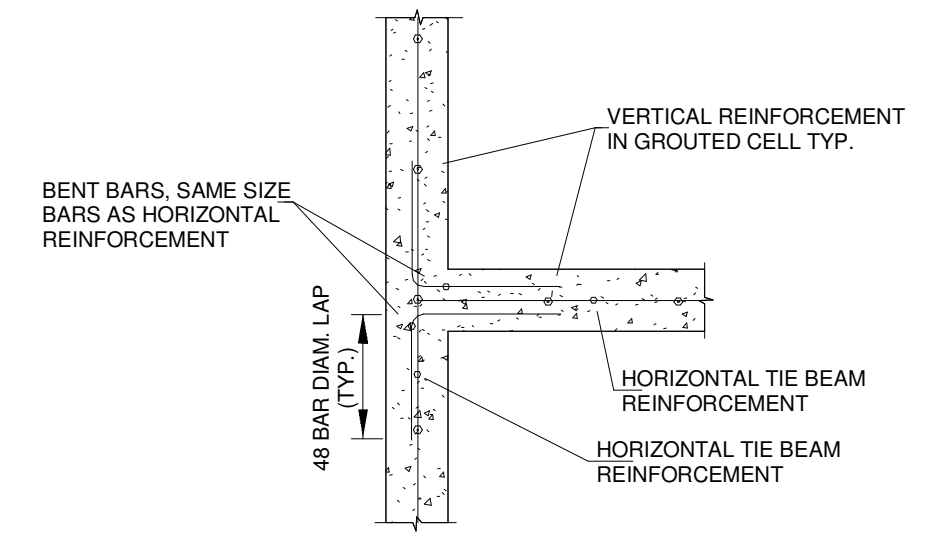
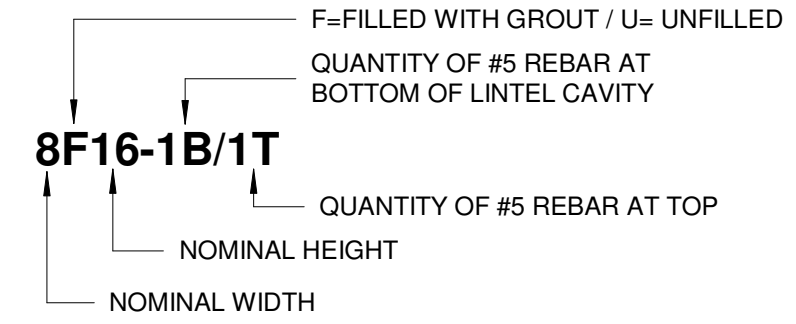
NOTE:
 ROOF VENTS SHOWN ARE BASED ON OFF RIDGE VENTS HAVING A MINIMUM NET FREE AREA OF 130 SQ. INCHES & PERFORATED SOFFIT MATERIAL HAVING A MIN. NET FREE OF 11.74 SQ. INCHES. CONTRACTOR REQUIRED TO VERIFY ALL ROOF VENTING PER THE CALCULATION IN FBC R806.2.

LOCATE ALL ROOF VENT PENETRATIONS MIN. 18" FROM RIDGES OR VALLEYS.

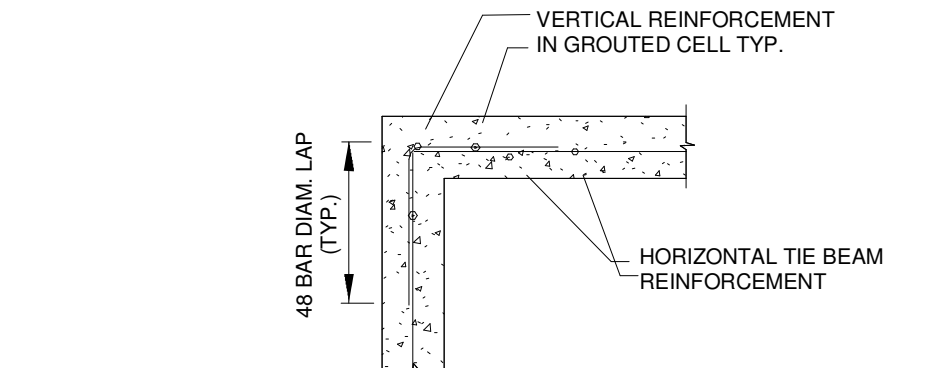
ROOF NOTES:

1. ROOF TRUSSES SHALL BE DESIGNED BY TRUSS MANUFACTURER. SHOP DRAWINGS SHALL BE SUBMITTED TO THE PROJECT ENGINEER FOR REVIEW PRIOR TO PRODUCTION.
2. TRUSS MANUFACTURED SHALL PROVIDE UPLIFT & REACTION VALUES FOR INDIVIDUAL TRUSSES. REFER TO THE TRUSS DRAWING FOR LAYOUT.
3. ROOF SHEATHING SHALL CONSIST OF 1/2" MIN. A.P.A. RATED SHEATHING. 4-PLY CDX LAID PERPENDICULAR TO TRUSSES NAILED @ 6" O.C. ALONG BOUNDARY EDGES, 6" O.C. ALONG EDGES AND 10" O.C. INTERMEDIATE W/ 8d COMMONS.
4. BRACE TRUSSES PER T.P.I. H.I.B.-91, AS REVISED.
5. PROVIDE SIMPSON HETA20 W/16 10d X 1 1/2" FOR UPLIFT UP TO 1890 LBS. PROVIDE (2) SIMPSON HETA20 @ OPPOSITE SIDE OF GIRDER TRUSS, UNLESS OTHERWISE NOTE.
6. ALL CHANGES OF THE TRUSS LAYOUT SHALL BE APPROVED BY THE ENGINEER.

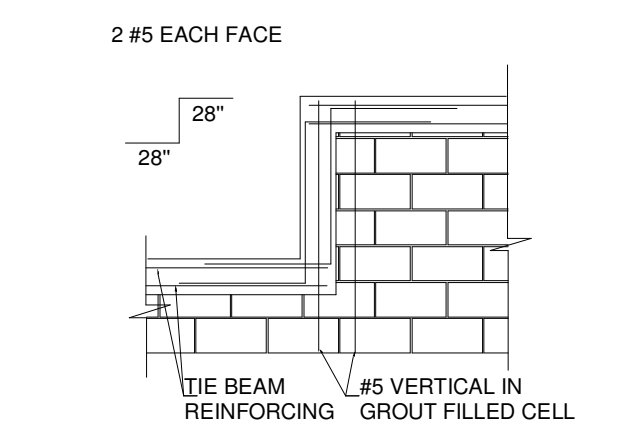
LINTEL & BOND BEAM LEGEND



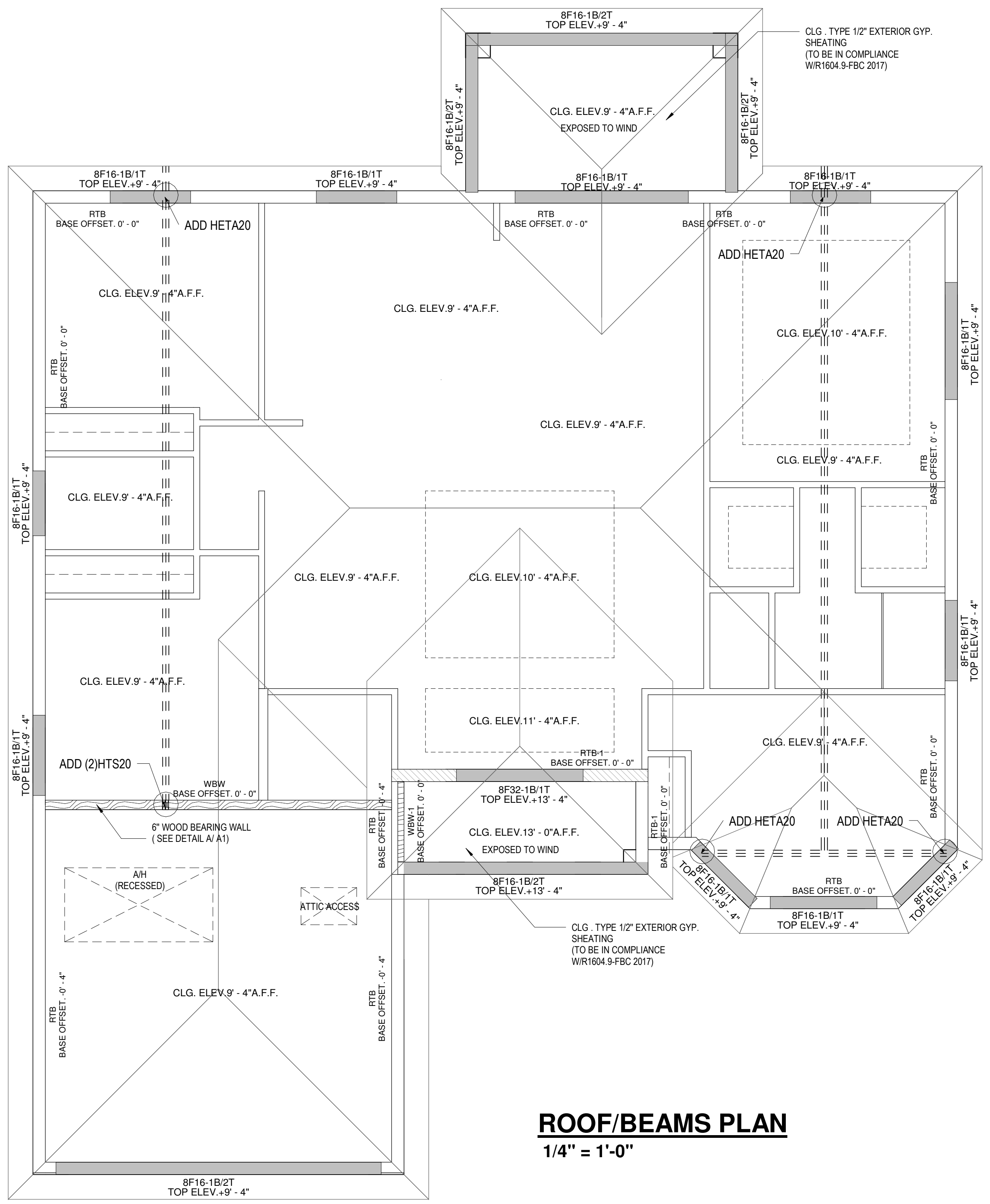
TYP. TIE BEAM INTERSECTION BENT AND LAP BAR DETAIL
 N.T.S.



TYP. TIE BEAM CORNER BENT AND LAP BAR DETAIL
 N.T.S.



DETAIL FOR RAISED BEAM
 N.T.S.

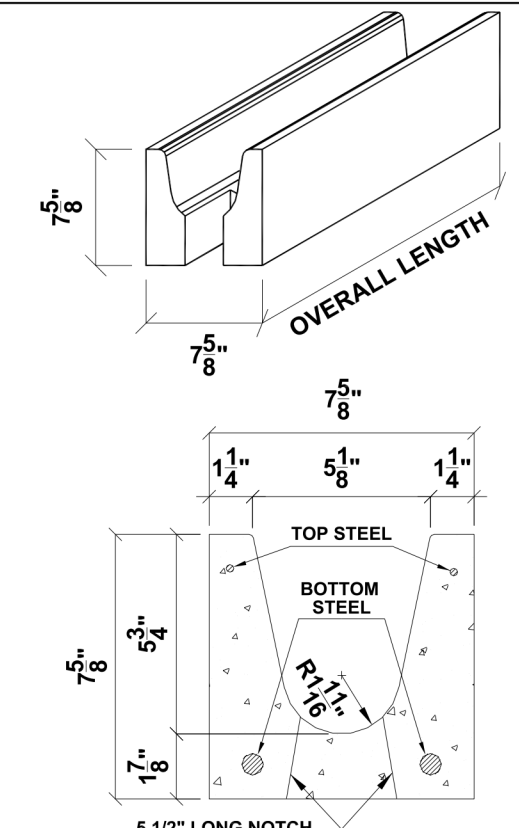


ROOF/BEAMS PLAN
 1/4" = 1'-0"

8" PRECAST U-LINTELS STANDARD LENGTHS

| OVERALL LENGTH | TOP STEEL | BOTTOM STEEL |
|----------------|---------------|--------------|
| 3'-0" (36") | 2 -7/32" wire | 2 -#3 rebar |
| 3'-4" (40") | 2 -7/32" wire | 2 -#3 rebar |
| 3'-6" (42") | 2 -7/32" wire | 2 -#3 rebar |
| 4'-0" (48") | 2 -7/32" wire | 2 -#3 rebar |
| 4'-6" (54") | 2 -7/32" wire | 2 -#3 rebar |
| 4'-8" (56") | 2 -7/32" wire | 2 -#3 rebar |
| 5'-4" (64") | 2 -7/32" wire | 2 -#3 rebar |
| 5'-10" (70") | 2 -7/32" wire | 2 -#3 rebar |
| 6'-0" (72") | 2 -7/32" wire | 2 -#4 rebar |
| 6'-6" (78") | 2 -7/32" wire | 2 -#4 rebar |
| 6'-8" (80") | 2 -7/32" wire | 2 -#4 rebar |
| 7'-4" (88") | 2 -7/32" wire | 2 -#4 rebar |
| 7'-6" (90") | 2 -7/32" wire | 2 -#4 rebar |
| 8'-0" (96") | 2 -#3 rebar | 2 -#4 rebar |
| 8'-8" (104") | 2 -#3 rebar | 2 -#4 rebar |
| 9'-4" (112") | 2 -#3 rebar | 2 -#4 rebar |
| 10'-0" (120") | 2 -#3 rebar | 2 -#4 rebar |
| 10'-6" (126") | 2 -#3 rebar | 2 -#4 rebar |
| 10'-8" (128") | 2 -#3 rebar | 2 -#5 rebar |
| 11'-4" (136") | 2 -#3 rebar | 2 -#5 rebar |
| 12'-0" (144") | 2 -#3 rebar | 2 -#5 rebar |
| 13'-4" (160") | 2 -#3 rebar | 2 -#5 rebar |
| 14'-0" (168") | 2 -#3 rebar | 2 -#5 rebar |

Additional lengths available by special order

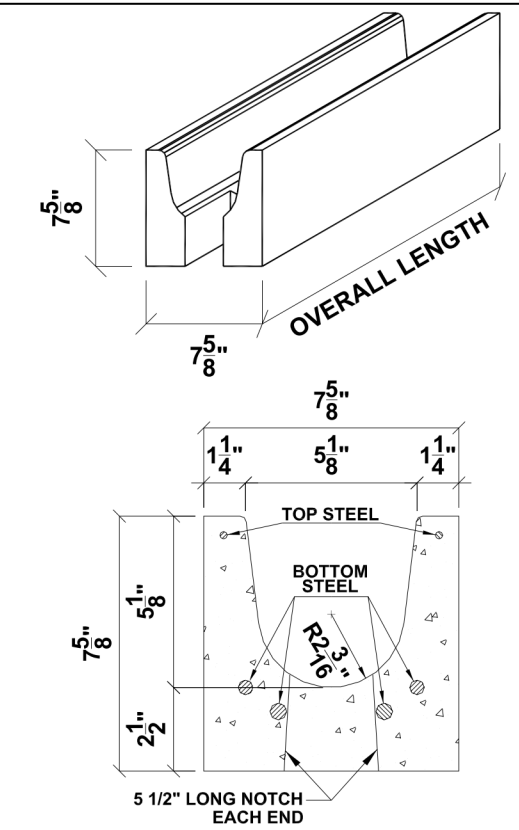


Rebar: ASTM A615 Grade 60
 Wire: ASTM A510
 Concrete Strength: 3500 psi
 Average Self Weight: 33 plf
 Finish: Grey Block

8" PRESTRESSED U-LINTELS STANDARD LENGTHS

| OVERALL LENGTH | TOP STEEL | BOTTOM STEEL |
|----------------|---------------|----------------|
| 14'-8" (176") | NONE | 2 -7/16 strand |
| 15'-4" (184") | NONE | 2 -7/16 strand |
| 17'-4" (208") | NONE | 2 -7/16 strand |
| 19'-4" (232") | 2 -7/32" wire | 2 -7/16 strand |
| 21'-4" (256") | 2 -7/32" wire | 2 -7/16 strand |
| 22'-0" (264") | 2 -7/32" wire | 2 -7/16 strand |
| 24'-0" (288") | 2 -7/32" wire | 2 -7/16 strand |

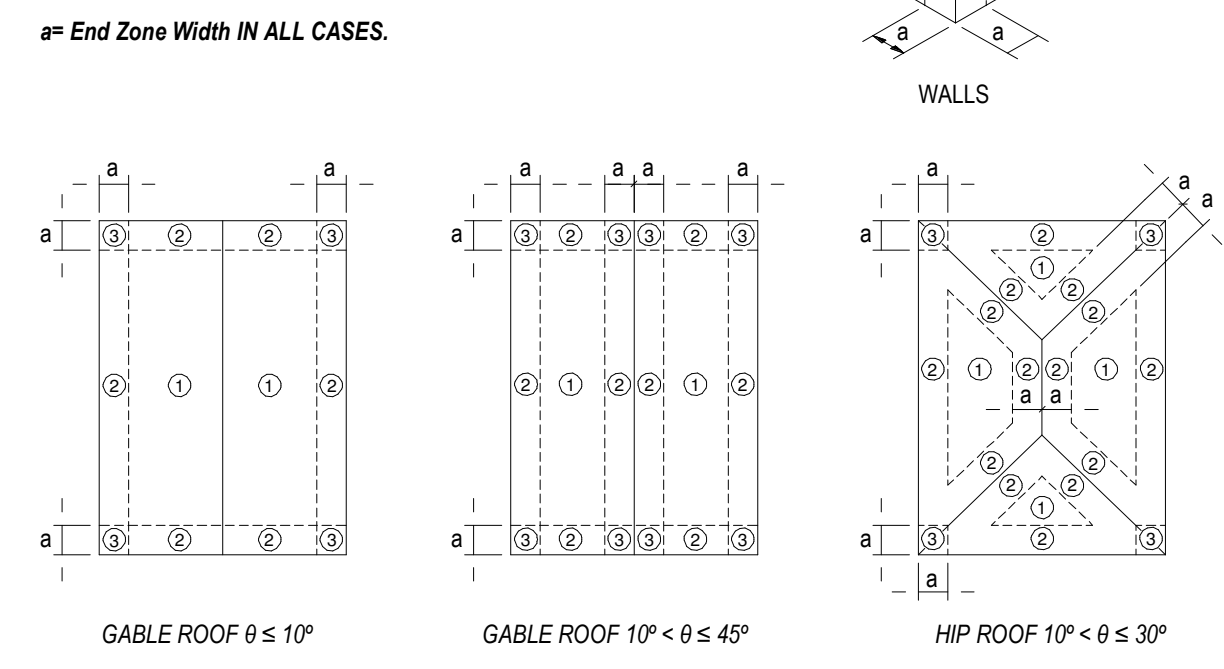
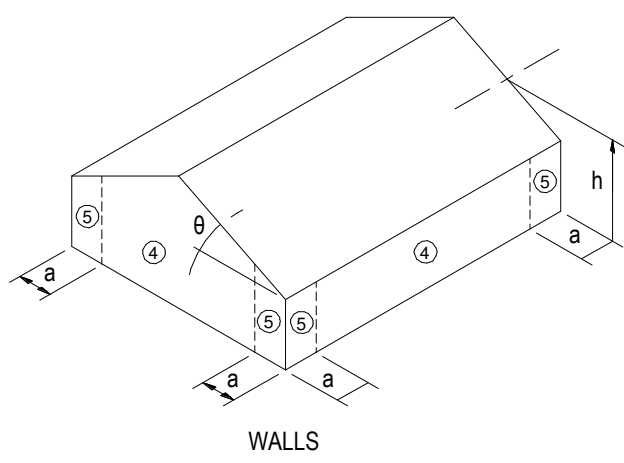
Additional lengths available by special order



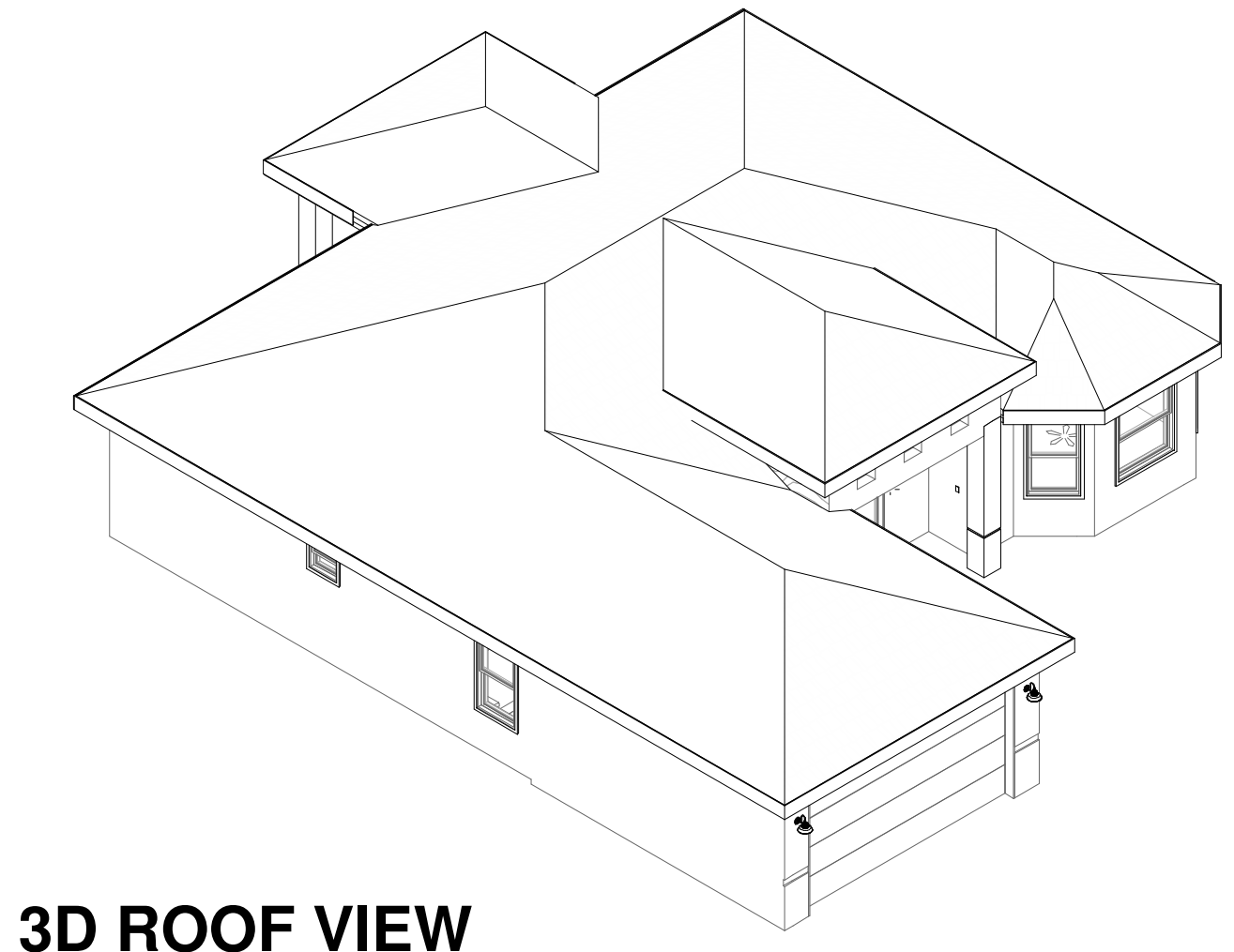
Rebar: ASTM A615 Grade 60
 Wire: ASTM A510
 Strand: ASTM A416 Grade 270
 Concrete Strength: 6000 psi
 Synthetic Fibers: 2.5 lbs/yd
 Average Self Weight: 37 plf
 Finish: Grey Smooth Form

COMPONENTS AND CLADDING

POSITIVE AND NEGATIVE PRESSURES PER TABLE R301.2(2) OF THE FLORIDA BUILDING CODE 6TH EDITION, 2017. RESIDENTIAL OVERHANG LOADS HAVE BEEN CALCULATED USING ASCE 7-10.



WINDOW AND DOOR DESIGN PRESSURES
 VALUES SHOWN ADJACENT TO EACH OPENING ON THE PLANS INDICATE WIND PRESSURE ON THAT PARTICULAR OPENING PLUS AND MINUS SIGNS SIGNIFY PRESSURE ACTING TOWARD AND AWAY FROM SURFACES RESPECTIVELY.



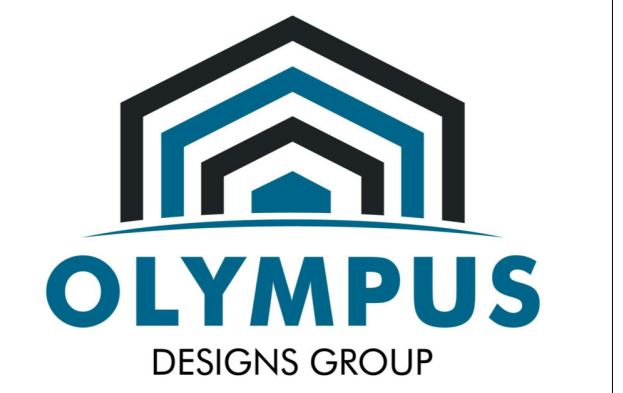
3D ROOF VIEW

ELEVATION SCHEDULE

| MARK | DESCRIPTION | TOP CONSTRAIN | BASE OFFSET |
|-------|----------------------------------|--------------------------------|-------------|
| RTB | (2)8" X 8" BOND BEAM W/(1)#5 T&B | Up to level: TOP OF B. BEAM | -0' - 4" |
| RTB | (2)8" X 8" BOND BEAM W/(1)#5 T&B | Up to level: TOP OF B. BEAM | 0' - 0" |
| RTB-1 | (2)8" X 8" BOND BEAM W/(1)#5 T&B | Up to level: TOP OF ENTRY BEAM | 0' - 0" |
| WBW | WOOD BEARING WALL | Up to level: TOP OF B. BEAM | 0' - 0" |
| WBW-1 | WOOD BEARING WALL | Up to level: TOP OF ENTRY BEAM | 0' - 0" |

PRECAST-LINTEL SCHEDULE

| DESCRIPTION | ELEVATION AT TOP | COUNT |
|-------------|------------------|-------|
| 8F16-1B/1T | 9' - 4" | 11 |
| 8F16-1B/2T | 9' - 4" | 4 |
| 8F16-1B/2T | 13' - 4" | 1 |
| 8F32-1B/1T | 13' - 4" | 1 |



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 5621 STRAND BLVD, SUITE 308
 NAPLES, FL 34110
 239 306-2324
 info@olympusdesigns.net

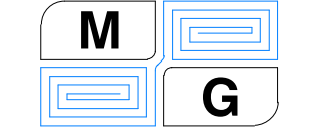
REVISIONS

| No. | Description | Date |
|-----|-------------|------|
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APPLICABLE CODES
 -6TH EDITION (2017) FLORIDA BUILDING CODE
 -6TH EDITION (2017) FLORIDA EXISTING CODE
 -2014 STANDARD NATIONAL ELECTRICAL CODE

DIGITAL SEAL

THIS DESIGN HAS BEEN PREPARED UNDER SUPERVISION, DIRECTION AND CONTROL OF:
 MG Engineering Consulting Services, Inc.
 3640 19th/ Ave SW
 Naples, FL 34117
 Phone: (239) 595 5465
 maxguerra@mg-engineering.us



MIRACLES LLC
 4311 15TH ST W LEHIGH ACRES FL 33971

ROOF/BEAMS PLAN

| | |
|----------------|-------|
| Project Number | |
| Date | 11/20 |
| Drawn by | IG |
| Notes | DS |

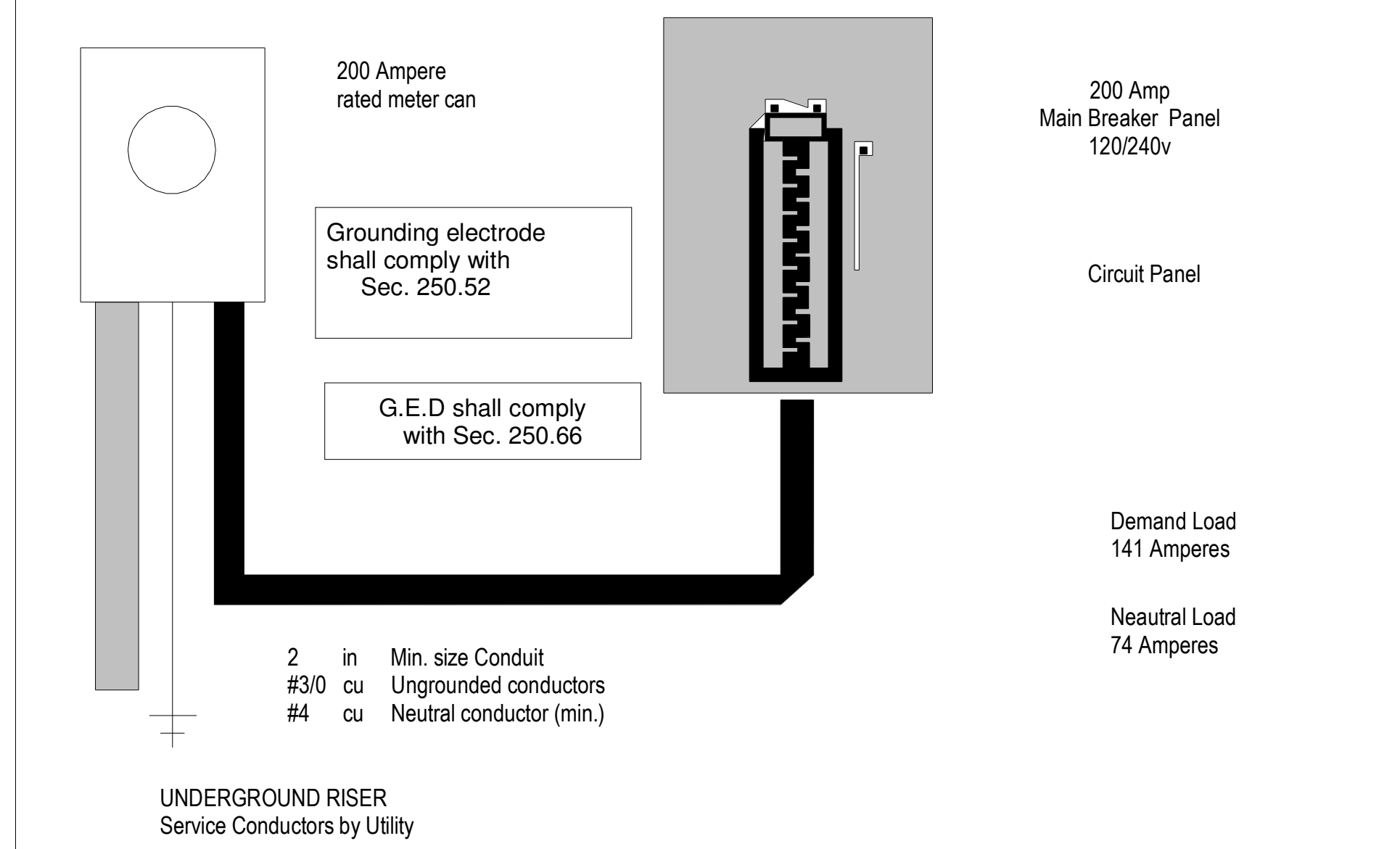
A4
 Scale As indicated

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ELECTRICAL LEGEND

| | |
|------|--|
| ⌘ | SINGLE SWITCH |
| ⌘₃ | THREE WAY SWITCH |
| ⌘ | DUPLEX RECEPTACLE |
| ⌘GFI | GFI RECEPTACLE |
| ⊙ | RECESSED LIGHT |
| ⊙ | EXHAUST FAN |
| SD | SMOKE DETECTOR |
| ▽ | TELEPHONE JACK |
| TV | T V / CABLE |
| ⊙ | CEILING LIGHT FIXTURE |
| ⊙ | WALL MOUNT LIGHT FIXTURE |
| ⊙ | WALL BRACKET LIGHT |
| ⊙ | VAPOR PROOF RECESSED LIGHT |
| ⊙ | EXTERIOR FLOOD LIGHT |
| ⊙ | PENDANT LIGHT |
| ⊙ | CEILING FAN W / LIGHTS |
| ⊙ | CEILING FAN W/O LIGHTS |
| ⊙ | PANEL BOX |
| ⊙ | FLOURESCENT |
| CM | CARBON MONOXIDE ALARM WITHIN 10' OF EACH BEDROOM |

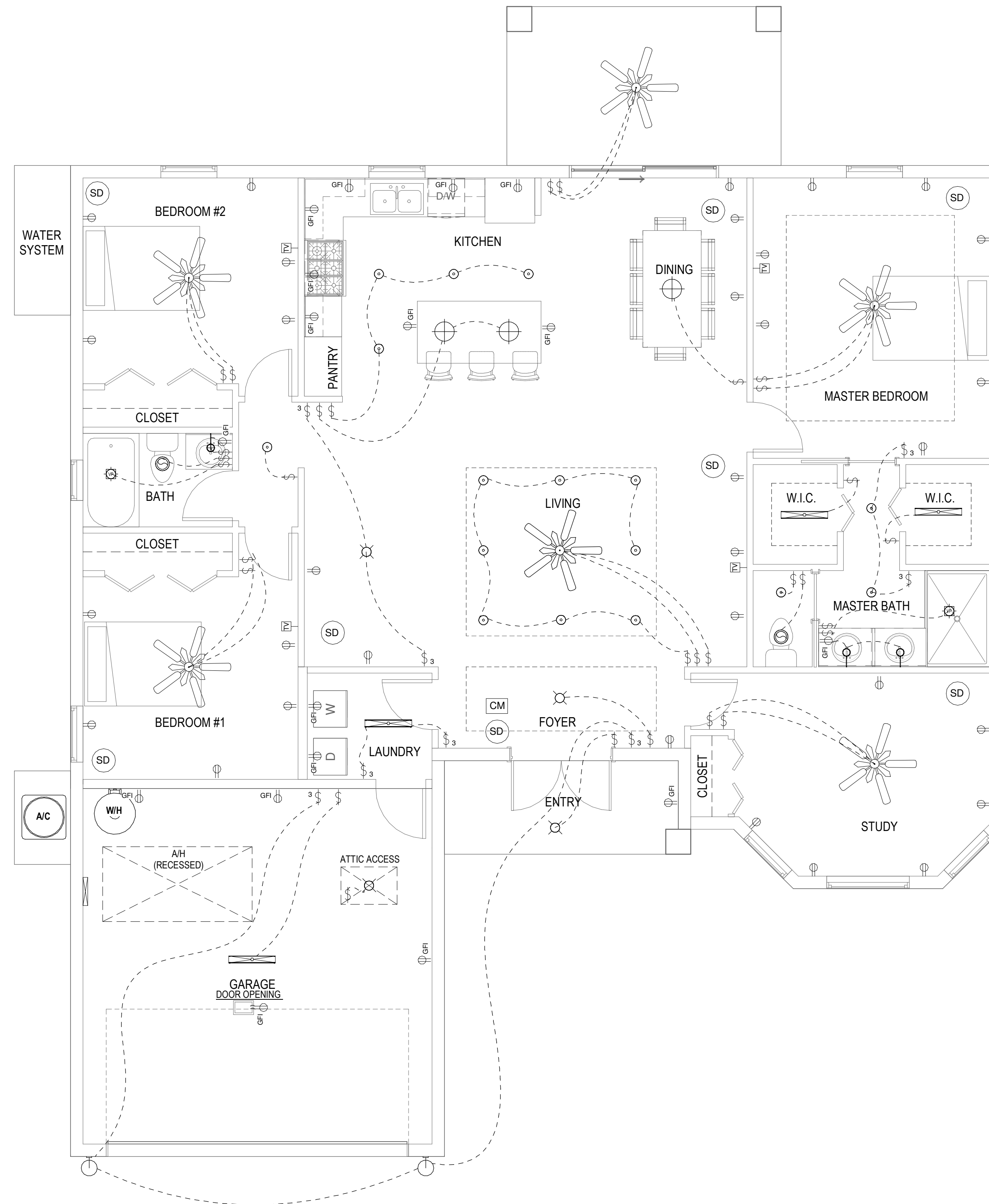
Residential Standard Calculation



NOTE:

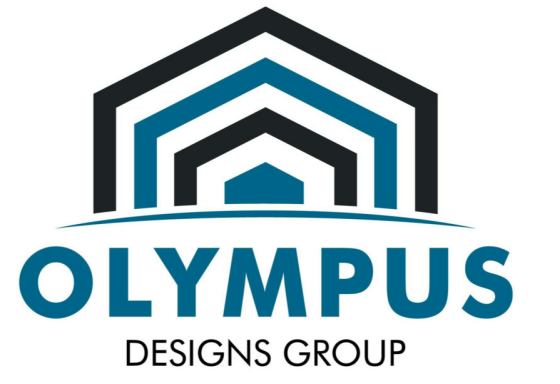
ALL BRANCH CIRCUITS THAT SUPPLY 125 V, SINGLE PHASE, 15 & 20 AMPERE RECEPTACLE OUTLET SHALL BE PROTECTED BY AN ARC-FAULT CIRCUIT INTERRUPTER(S) IN DWELLING UNIT ALL LIVING AREA.

2014 N.E.C/6TH EDITION F.B.C 2017



ELECTRICAL PLAN

1/4" = 1'-0"



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NAPLES, FL 34110
239 306-2324
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REVISIONS

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ELECTRICAL PLAN

| | |
|----------------|-------|
| Project Number | |
| Date | 11/20 |
| Drawn by | IG |
| Notes | DS |

A5

Scale As indicated

GENERAL STRUCTURAL NOTES:

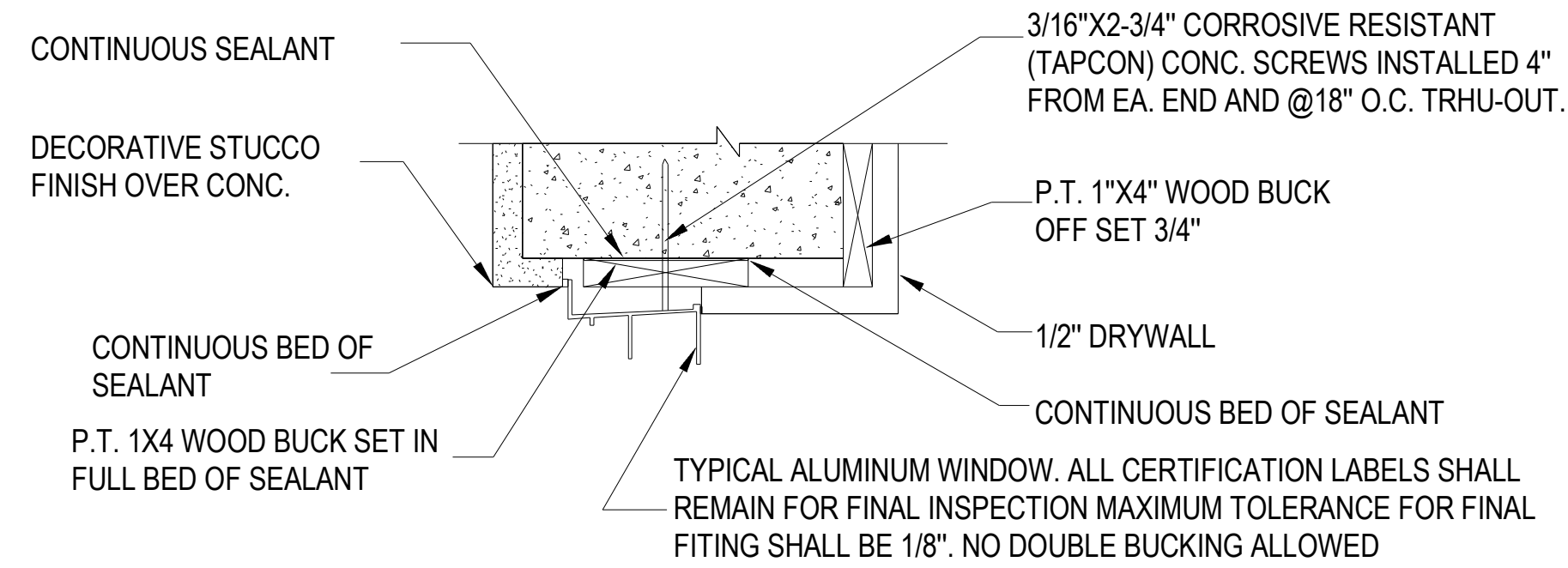
- FOUNDATIONS:**
SOIL STATEMENT- VISUAL INSPECTION OF THE SITE SHOWS A SOIL OF LIMESTONE WITH SANDY POCKETS AND WITH A LAYER OF TOPSOIL. THIS LIMESTONE HAS A SAFE BEARING CAPACITY IN EXCESS OF 2,000 psf. AT THE TIME OF CONSTRUCTION A LICENSED ARCHITECT OR PROFESSIONAL ENGINEER SHALL SUBMIT TO THE BUILDING OFFICIAL A LETTER ATTESTING THAT THE SITE HAS BEEN OBSERVED AND THE FOUNDATIONS ARE SIMILAR TO THOSE UPON WHICH THE DESIGN IS BASED.
- CONCRETE:**
ALL CONCRETE TO ATTAIN A MINIMUM ULTIMATE COMPRESSIVE STRENGTH OF 3,000 PSI IN 28 DAYS. AGGREGATES TO BE CLEAN AND WELL GRADED, MAXIMUM SIZE 1". CONCRETE SLUMP: 3" MIN. TO 5" MAX. VERTICAL CONCRETE DROP NOT TO EXCEED 8'.
- CONCRETE COVER:**
TO BE AS FOLLOWS:

| | |
|----------------------------------|-----------------|
| FOOTINGS | 3" |
| BEAMS | 1.5" TO STIRRUP |
| COLUMNS | 1.5" TO TIES |
| SLABS NOT EXPOSED TO THE WEATHER | 0.75" |
| SLABS EXPOSED TO THE WEATHER | 1.5" |
- REINFORCING STEEL:**
TO BE NEW HIGH STRENGTH BILLET STEEL DEFORMED AS PER ASTM A-305, AND CONFORMING TO ASTM A-615, GRADE 60. LAP CONTINUOUS BARS 30-BAR DIAMETERS, UNLESS OTHERWISE NOTED IN PLANS. HOOK DISCONTINUOUS ENDS OF ALL TOP BARS. REINFORCING STEEL TO BE DETAILED AND FABRICATED IN ACCORDANCE WITH "MANUAL OF STANDARD PRACTICE OF DETAILING REINFORCING CONCRETE STRUCTURES", AND THE ACI BUILDING CODE 318, LATEST EDITION. SUBMIT SHOP DRAWINGS FOR APPROVAL BY ENGINEER BEFORE FABRICATION.
- MASONRY:**
REINFORCED MASONRY DESIGN & CONSTRUCTION SHALL COMPLY W/ ACI 530-02/ASCE 5-02/TMS 402-02 & ACI 5301.-02/ASCE 6-02/TMS 602-02.
 - ALL CONCRETE BLOCK TO BE GRADE N-2, CONFORMING TO ASTM C- 90, WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 1,900 PSI, AND A PRISM STRENGTH OF 1,500 PSI (MINIMUM). MORTAR SHALL BE TYPE M, WITH A MINIMUM STRENGTH OF 2,500 PSI (USE PORTLAND TYPE CEMENT).
 - MASONRY WALLS SHALL BE REINFORCED HORIZONTALLY EVERY OTHER COURSE WITH 9 GAUGE DEFORMED GALVANIZED STEEL LADDER.
 - FOR VERTICAL REINFORCEMENT, SEE PLAN AND LAP 48 BAR DIAMETERS MINIMUM. PROVIDE FULL BED OF MORTAR FOR REINFORCED MASONRY. GROUTING OF CELLS SHALL BE A CONTINUOUS OPERATION IN LIFTS NOT EXCEEDING 4'-0" AND A MAXIMUM POUR OF 12'-0" IN HEIGHT. GROUTING TO BE CONSOLIDATED BETWEEN LIFTS BY PUDDLING, RODDING OR MECHANICAL VIBRATION. PROVIDE CLEANOUTS, AND CLEAN OUT CELLS OF MORTAR DROPPINGS.
 - GROUT SHALL BE A PLASTIC MIX HAVING A MAXIMUM SLUMP OF 8" TO 10", PLACED BEFORE ANY INTIAL SET HAS OCCURRED, BUT IN NO CASE MORE THAN 1-1/2 HOURS AFTER THE MIX-DESIGN WATER HAS BEEN ADDED, GROUT TO ATTAIN A MINIMUM 3,000 PSI COMPRESSIVE STRENGTH.
- PRE-FABRICATED WOOD TRUSSES:**
SHALL BE DESIGNED AND FABRICATED IN ACCORDANCE WITH THE TRUSS PLATE INSTITUTE (T.P.I), LATEST EDITION. TRUSSES TO BE ERCTED FOLLOWING THE GUIDELINES OF THE T.P.I PUBLICATION BWT-76. SUBMIT SHOP DRAWINGS W/ LAYOUT PLAN AND INDIVIDUAL DRAWINGS FOR EACH DIFFERENT TRUSS, SIGNED AND SEALED BY A FLORIDA REGISTERED PROFFESIONAL ENGINEER, FOR APPROVAL BY STRUCTURAL ENGINEER PRIOR TO FABRICATION.

GENERAL CONTRACTOR TO PROVIDE PERMANENT LATERAL BRACING OF THE BOTTOM CHORD AND THE WEB MEMBERS IN ACCORDANCE WITH THE RECOMMENDATIONS OF T.P.I BWT-76 AND THE REQUIREMENTS OF THE INDIVIDUAL TRUSS DESIGNS. SUBMIT PERMANENT BRACING PLAN FOR APPROVAL BY ENGINEER PRIOR TO ERECTION. GRIDER TRUSSES TO BE BOLTED TOGETHER WITH 1/2" DIA. BOLTS AT 24" C/C AT TOP CHORD, BOTTOM CHORD, AND WEB MEMBERS. CONTRACTOR TO SUBMIT SIGNED AND SEALED SHOP DRAWINGS BY A DELEGATED SPECIALTY ENGINEER FOR ALL TRUSS TO TRUSS CONNECTIONS. TRUSS MANUFACTURER TO VERIFY UPLIFT AND GRAVITY TRUSS REACTIONS SHOWN ON FRAMING PLANS.

SUPERIMPOSED LOADS SHALL BE AS FOLLOWS:

| | |
|-------|-------------------------------|
| ROOF: | DEAD LOAD= 20 PSF (TOP CHORD) |
| | 10 PSF (BOTT.CHORD) |
| | LIVE LOAD= 20 PSF (TOP CHORD) |
- WIND DESIGN CRITERIA:**
ALL STRUCTURAL ELEMENTS, EXPOSED TO WIND, HAVE BEEN DESIGNED PER THE GUIDELINES OF THE HVHZ-FBC 6TH EDITION 2017, ASCE 7-10: RISK CATEGORY II, ENCLOSE BUILDING, 160 MPH WIND SPEED, EXPOSURE B. SEE CALCULATIONS FOR ADDITIONAL INFORMATION.
- GENERAL:**
THE BUILDING WAS DESIGNED ACCORDING TO THE FLORIDA BUILDING CODE 6TH EDITION 2017. STRUCTURAL DRAWINGS SHALL BE WORKED TOGETHER WITH ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS TO LOCATE DEPRESSED SLABS, SLOPES, OPENINGS AND DIMENSIONS, ETC. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT BEFORE PROCEEDING WITH THE WORK. CONTRACTOR TO VERIFY ALL DIMENSIONS AND CONDITIONS AT JOB SITE BEFORE STARTING WORK. CONTRACTOR IS RESPONSIBLE FOR ALL SHORING AND RE-SHORING REQUIRED.

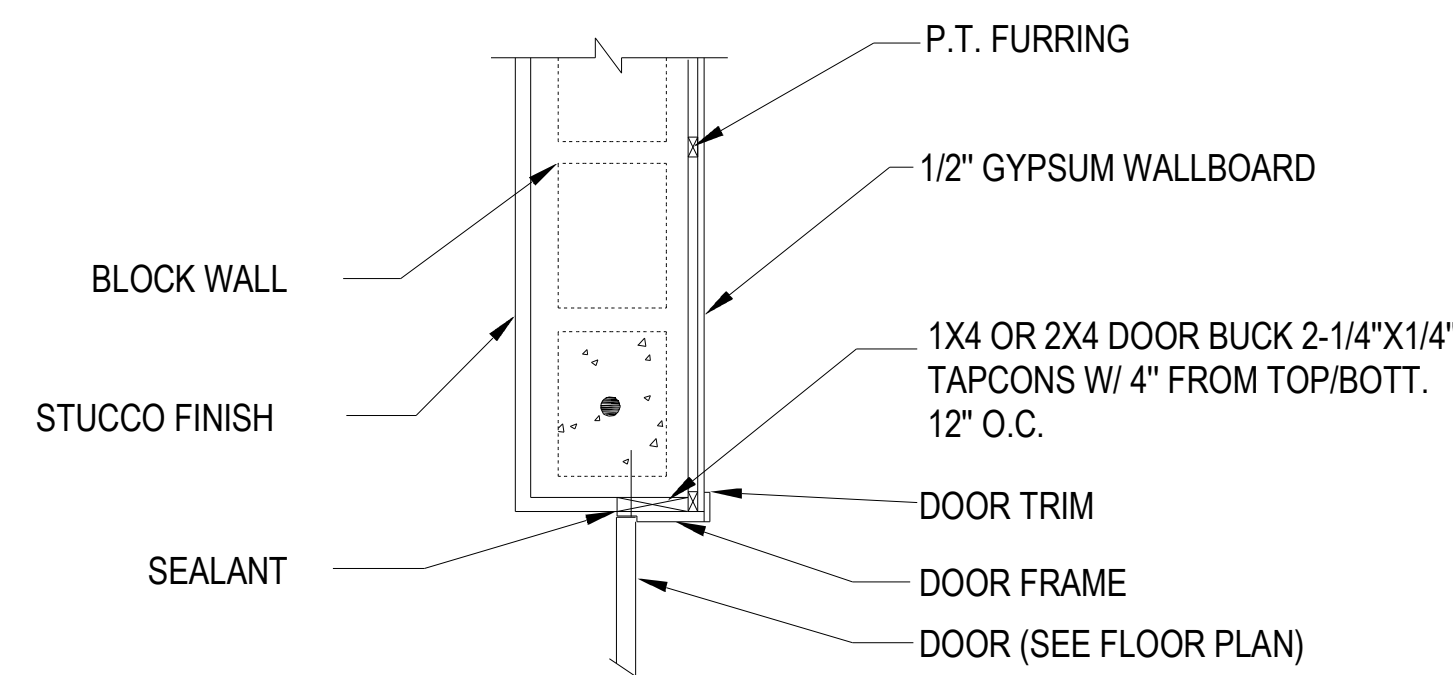


WINDOW HEAD AND JAMB DETAIL

SCALE: N.T.S.

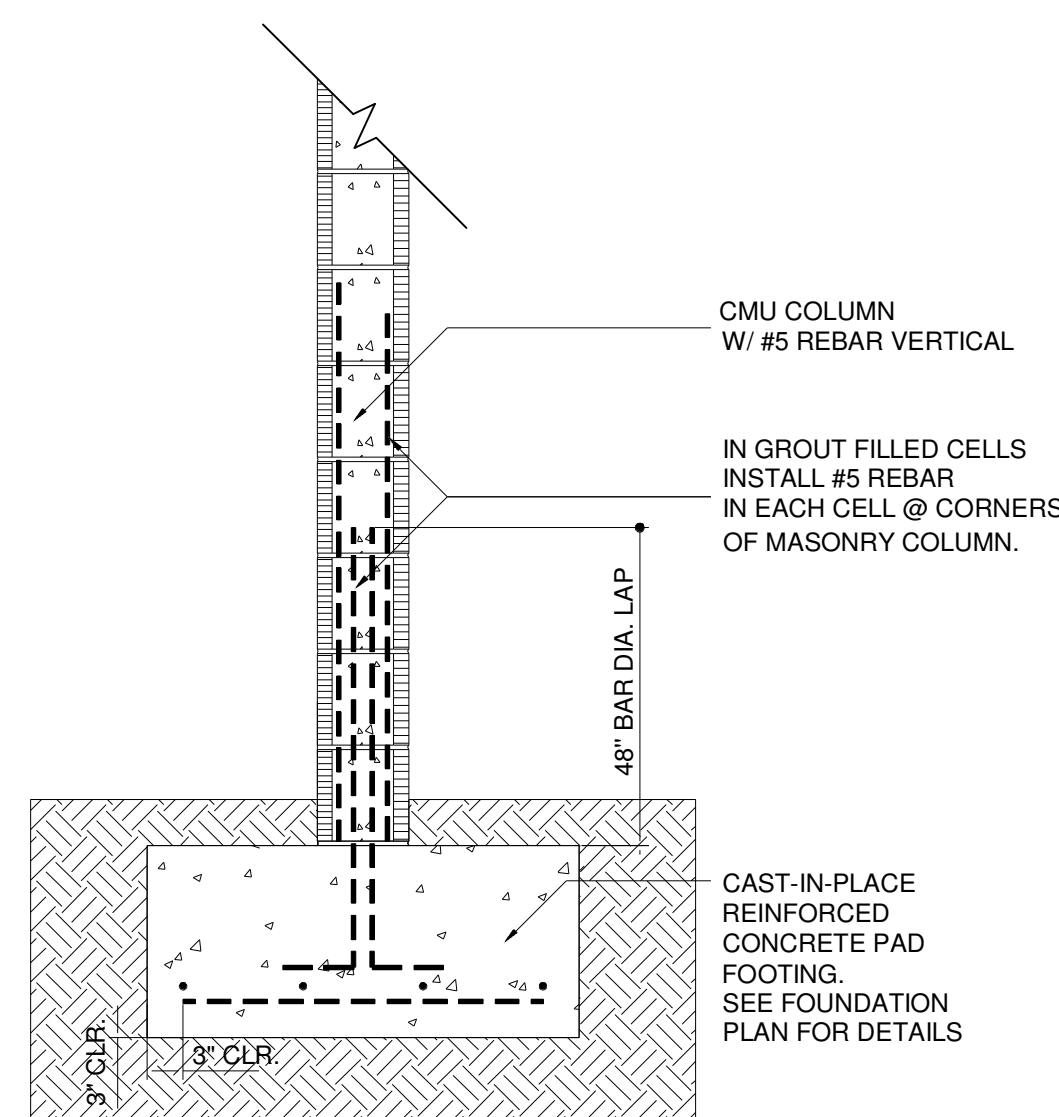
NOTES:

- FOR WINDOW AND DOOR ATTACHMENT TO MASONRY USE BUCK IN BED OF LIQUID NAIL TO MASONRY SIDE AND TOP OF OPENING WITH (1) 3/4" X .097" POWER DRIVEN T-NAIL OR 3/16" X 2-1/4" TAPCONS. START AT 4" THEN 8" ON CENTER. ATTACH WINDOW IN ACCORDANCE WITH WINDOW MANUFACTURER'S SPECIFICATIONS.
- FOR WINDOW ATTACHMENT TO FRAME USE MINIMUM (4) 8D NAILS PER EDGE, THEN 12" ON CENTER MAXIMUM.
- FOR DOOR ATTACHMENT TO FRAME USE No.8 x 3" STEEL SCREWS. (2) AT HEADER, (3) AT LOCK SIDE AND (2) MINIMUM EACH HINGE.
- CONTRACTOR AND OR SUB-CONTRACTOR TO REVIEW THE TRUSS PLACEMENT PLAN TO COORDINATE.



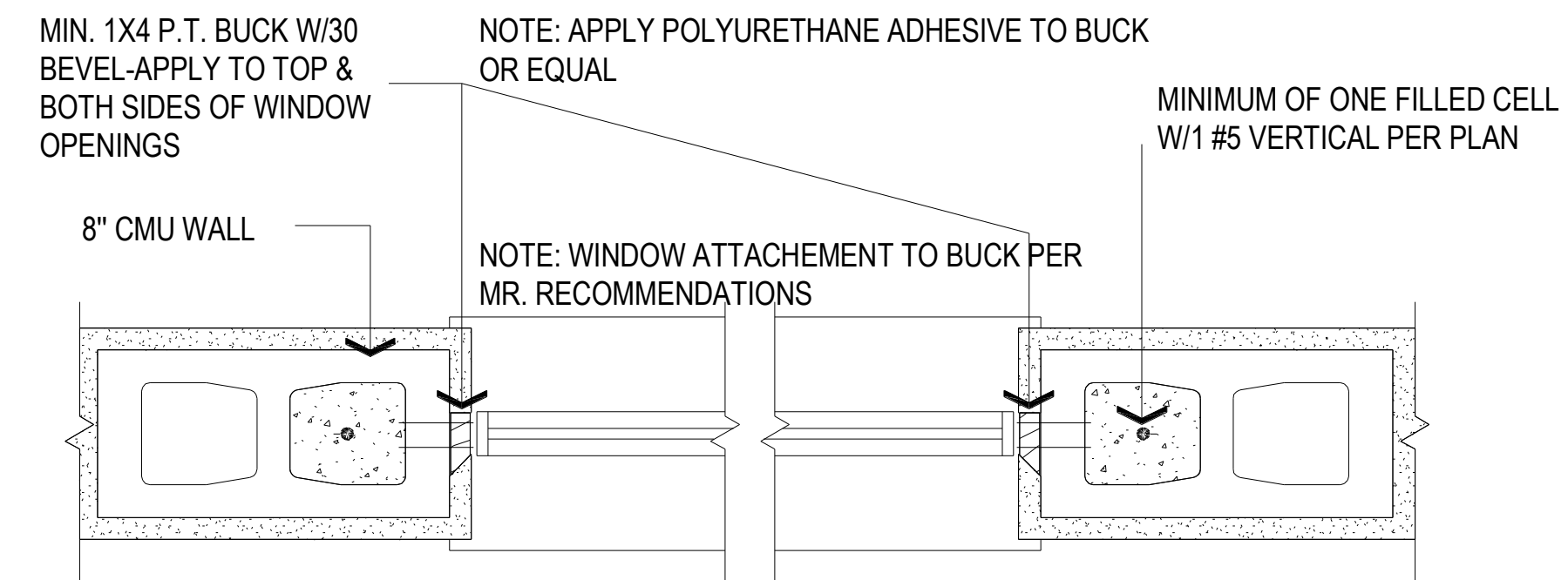
TYPICAL EXTERIOR DOOR DETAIL

SCALE: N.T.S.



TYP. CMU COLUMN DETAIL

SCALE: N.T.S.

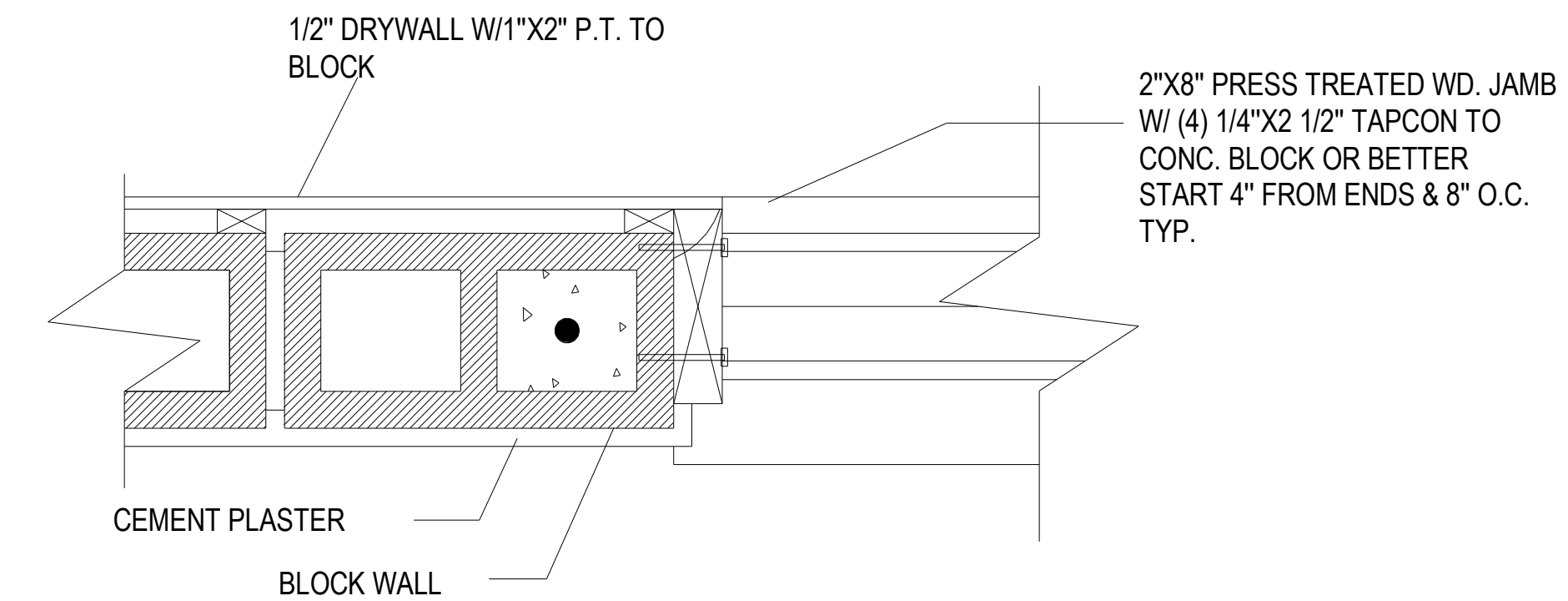


BUCK FASTENERS TO MASONRY TO BE ONE OF THE FOLLOWING:

- 1/4" X 3" TAPCONS @ 16" O.C. (MIN. 3 PER BUCK)
- 10D FULL HEAD COIL NAIL @ 12" O.C.
- 8D FULL HEAD COIL NAIL @ 8" O.C.
- 6D FULL HEAD COIL NAIL @ 6" O.C.

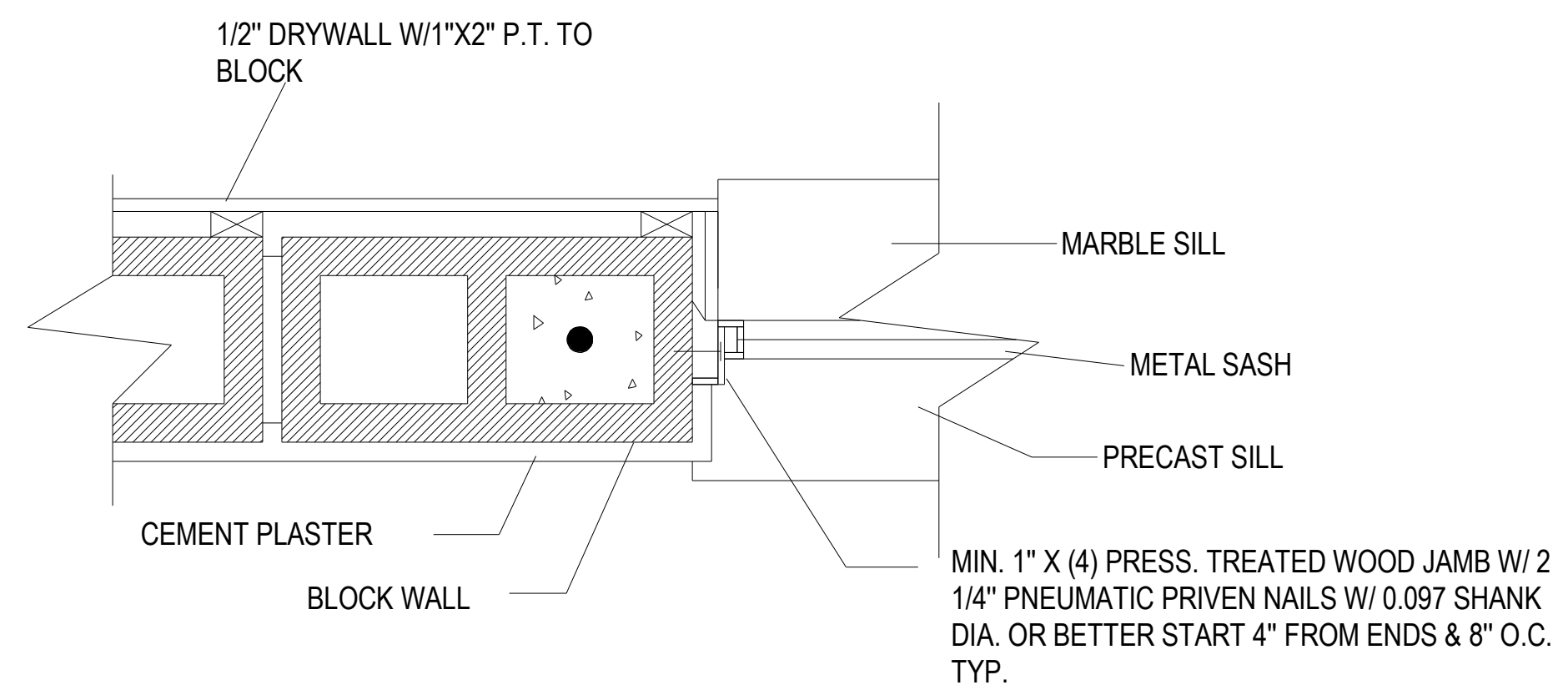
WINDOW INSTALLATION DETAIL

SCALE: N.T.S.



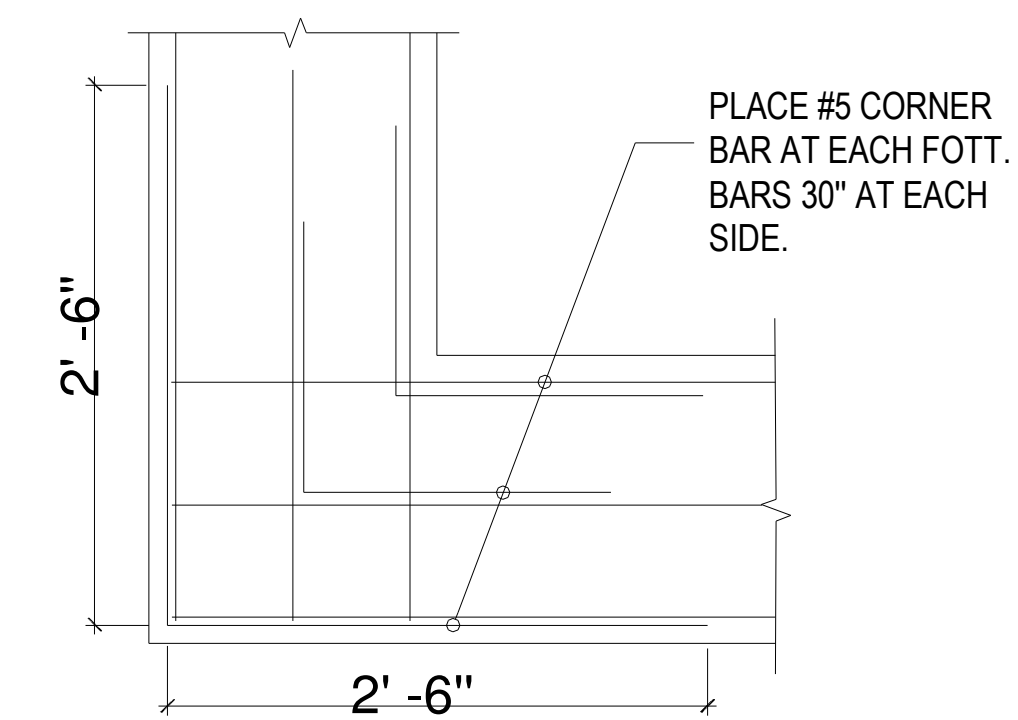
SL. GR. DR. JAMB TO BLK DETAIL

SCALE: N.T.S.



DOOR AND WINDOW JAMB DETAIL

SCALE: N.T.S.



TYP. FOOTING CORNER BAR DETAIL

SCALE: N.T.S.



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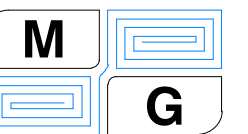
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A6

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